



PŘÍLOHA C:

MANUÁL A SOFTWARE MODELU KINFIL CERTIFIKOVANÁ METODIKA 2015 TAČR TA02020402

PROF. ING. P. KOVÁŘ, DRSC.

ING. DARINA HEŘMANOVSKÁ, PH.D.

**KATEDRA BIOTECHNICKÝCH ÚPRAV KRAJINY, FAKULTA ŽIVOTNÍHO PROSTŘEDÍ NA ČESKÉ
ZEMĚDĚLSKÉ UNIVERZITĚ V PRAZE**

září 2015

Obsah

1. Úvod	3
2. Infiltrační přístup	4
3. Transformace přímého odtoku	5
4. Struktura modelu KINFIL.....	6
5. Literatura	8
6. Software - zdrojový program modelu KINFIL (FORTRAN).....	19

Anotace

Model KINFIL je určen pro stanovení návrhových průtoků ovlivněných antropogenní činností, jako např. změna kultur, odlesnění nebo urbanizace, a simulaci významných odtokových procesů.

1. Úvod

Model KINFIL je založen na kombinaci teorie infiltrace a transformace přímého odtoku kinematickou vlnou (Overton, Meadows, 1976; Stephenson, Meadows, 1986; Beven, 1986, 2006). Osvědčil se na řadě experimentálních povodí při rekonstrukci historických povodňových případů a osvědčuje se, podobně jako ostatní modely kinematické vlny, v řadě případů simulace povrchového odtoku, způsobujícího erozi půdy (Kovář, 1992, 2000). Tento model používá fyzikálně-geometrické (fyziografické), hydraulické a klimatické parametry povodí, které se dají určit z mapových a jiných podkladů při absenci přímých pozorování a při zohlednění důsledku antropogenní činnosti v povodí (Morgan, Nearing, 2011). Model je určen přednostně pro stanovení návrhových průtoků pro různé „scénářové situace dané touto činností, jako je změna kultur, odlesnění, urbanizace aj. Současná verze modelu KINFIL je založena na infiltrační teorii Greena a Ampta se zavedením koncepce výtopy podle Meina a Larsona (1973) a Morel-Seytoux (Morel-Seytoux, Verdin, 1981; Morel-Seytoux, 1982):

$$K_s \left(\frac{z_f + H_f}{z_f} \right) = (\theta_s - \theta_i) \frac{dz_f}{dt} \quad (1)$$

$$S_f = (\theta_s - \theta_i) \cdot H_f \quad (2)$$

$$t_p = \frac{S_f}{i \left(\frac{i}{K_s} - 1 \right)} \quad (3)$$

kde K_s je nasycená hydraulická vodivost (m/s), z_f hloubka infiltrační fronty (m), θ_s nasycená půdní vlhkost (-), θ_i počáteční půdní vlhkost (-), H_f sací tlak pod infiltrační frontou (m), i intenzita deště (m/s), S_f retenční součinitel sacího tlaku (m), t_p doba výtopy (s) a t čas (s).

Základním úkolem je určení parametru nasycené hydraulické vodivosti K_s a retenčního součinitele sacího tlaku S_f (při stavu polní vodní kapacity – PVK). Přímým řešením na malých je experimentálních plochách je měření těchto parametru. Na větších povodích je to pak využití dříve odvozených vztahu mezi těmito parametry a hodnotami čísel odtokových křivek CN (Curve Number), dnes dobře propracované metody a ve světě široce používané (US SCS, 1972, 1986). Indexové hodnoty CN korespondují s konceptuálními hodnotami půdních parametru K_s a S_f (PVK): $CN = f(K_s, S_f)$. Druhým komponentem modelu KINFIL je jeho část simulující propagaci a transformaci přímého odtoku (Beven, 2006). Řešená parciální diferenciální rovnice popisuje neustálený pohyb, aproximovaný kinematickou vlnou (po zanedbání nevýznamných rychlostních členu dynamické St. Venantovy rovnice) po ploše různě uspořádaných a podle topografických podmínek různě skloněných rovinných desek:

$$\frac{\partial y}{\partial t} + \alpha y^{m-1} \frac{\partial y}{\partial x} = i_e(t) \quad (4)$$

kde x, y, t jsou souřadnice délky, hloubky, času (m, m, s), α, m hydraulické parametry a $i_e(t)$ je intenzita efektivního deště (m/s).

Tato rovnice je převedena do tvaru konečných diferencí a řešena explicitním numerickým schématem. Pro praktické řešení je povodí geometrizováno rozdělením do tří komponent: kaskády desek, konvergentních a divergentních segmentů a úseku koryta toku tak, aby simulace topografických ploch povodí byla dostatečně reprezentativní. Počáteční podmínky řešení diferenčního schématu jsou zadány pro tzv. nulové hodnoty hloubek vody (tj. jestliže $y(x, 0) = 0$ pro všechny souřadnice polohy x). Horní okrajová podmínka je dána polohou každé rovinné desky v kaskádě, případně horní hranou segmentu. Pro soustředěné neustálené proudění v korytě bývá používáno submodelu Muskingum-Cunge (Cunge, 1969), jehož autoři zavedli zjednodušující předpoklady do rovnice kinematické vlny transformované korytem toku.

2. Infiltrační přístup

Řešení vychází z teorie Greena a Ampta v úpravě Morel-Seytoux, založené na výpočtu tzv. doby výtopy t_p . V rovnicích se uplatňují dva parametry:

- nasycená hydraulická vodivost K_s (m/s),
- retenční součinitel sacího tlaku S_f (m): $S_f = (\theta_s - \theta_i) \cdot H_f$, kde θ_s je vlhkost pudy při nasycení (-), θ_i počáteční vlhkost (-), θ_{FC} vlhkost při polní vodní kapacitě (-) a H_f sací tlak na infiltrační frontě.

Sorptivita pudy při polní vodní kapacitě (m/s^{0,5}) je pak dána vztahem:

$$S(\theta_{FC}) = S(\theta_{FC}) = \sqrt{2K_s \cdot S_f} \quad (5)$$

Z rovnice Greena a Ampta:

$$v_f = K_s \cdot \left[i + \frac{(\theta_s - \theta_i) \cdot H_f}{W} \right] \quad (6)$$

kde W je kumulativní infiltrace (m), a z teorie Meina a Larsona (1973) o stanovení doby výtopy byly Morel-Seytouxem odvozeny infiltrační rovnice (Morel-Seytoux, 1976). Pro dešť o konstantní intenzitě se kumulativní infiltrace W v case $t_p < t < t_D$, vypočítá ze vztahu:

$$W = W_p + S(\theta_i)AR \left[\sqrt{t - t_p + \frac{t_p}{2}(AR)^3} - \sqrt{\frac{t_p}{2}(AR)^3} \right] + K_s(t - t_p) \quad (7)$$

kde

$$AR = \frac{i_+}{i_+ - 1}, \quad i_+ = \frac{i}{K_s}, \quad W_p = i \cdot t_p \quad (8)$$

Počátek výtopy ($t = t_p$) pro dešť s konstantní intenzitou se stanoví z podmínek (Mein, Larson, 1973) $\theta_i \rightarrow \theta_s$ a $v_f = i$. Kumulativní infiltrace v době výtopy t_p je pak:

$$W_p = v_f \cdot t_p = i_p \cdot t_p \quad (9)$$

kde i_p je intenzita deště v době výtopy (pro dešť s konstantní intenzitou $i_p = i$).

Obdobně pro případ deště s proměnlivou intenzitou platí:

$$W = W_p + S(W_p, \theta_i) + \left[\sqrt{t - t_p + BR} - \sqrt{BR} \right] + K_s(t - t_p) \quad (10)$$

kde

$$S(W_p, \theta_i) = \sqrt{\frac{2K_s(S_F + W_p)^2}{S_F}} \quad (BR) = \frac{1}{2} \frac{(S_F + W_p)^2}{K_s S_f \left(\frac{i_p}{K_s} - 1\right)} \quad (11)$$

3. Transformace přímého odtoku

Model kinematické vlny je model s rozdělenými parametry (distributed model), je možné ho použít na různých geometrických útvarech, jako:

- kaskáda rovinných desek,
- konvergentní nebo divergentní segmenty,
- soustředěný odtok v úsecích říčních koryt.

Proudění vody po přímém nepropustném svahu simulovaného širokou rovinnou deskou (resp. konvergentním segmentem) je možné vyjádřit rovnicí kinematické vlny jako

$$\frac{\partial y}{\partial t} + \frac{\partial q}{\partial x} = i_e(t) \left(+ \frac{q}{L-x} \right) \quad (12)$$

$$q = \alpha \cdot y^m \quad (13)$$

kde q je průtok na jednotkovou šíři svahu (m^2/s), $i_e(t)$ je laterální přítok, neboli intenzita, efektivního deště (m/s), α , m jsou hydraulické parametry, L je poloměr konvergentního segmentu, t a x jsou souřadnice času (s) a polohy (m). Spojením rovnic dostáváme

$$\frac{\partial y}{\partial t} + m\alpha y^{m-1} \frac{\partial y}{\partial x} = i_e(t) \left(+ \frac{\alpha y^m}{L-x} \right) \quad (14)$$

Soustředěný odtok v říčních korytech je řešen metodou Muskingum-Cunge (Cunge, 1969):

$$K \frac{d}{dt} (XQ_j) + (1-X)Q_{j+1} = Q_j - Q_{j+1} \quad (15)$$

Model kinematické vlny podle explicitního numerického schématu má řešení (Lax, Wendroff, 1960) pro hloubky vodního proudu:

$$\begin{aligned} y_j^{i+1} = & y_j - \frac{\Delta t}{2\Delta x} \cdot (\alpha y_{j+1}^m - \alpha y_{j-1}^m - 2\Delta x (i_e)_j) \\ & + \frac{(\Delta t)^2}{4(\Delta x)^2} \cdot (\alpha y_{j+1}^{m-1} + \alpha y_{j-1}^{m-1}) (\alpha y_{j+1}^m - \alpha y_j^m - \Delta x \cdot (i_e)_j) \\ & - \frac{(\Delta t)^2}{4(\Delta t)^2} \cdot (\alpha y_j^{m-1} + \alpha y_{j-1}^{m-1} - \Delta x \cdot (i_e)_j) + \frac{\Delta t}{2} [(i_e)_j^{i+1} - (i_e)_j] \end{aligned} \quad (16)$$

V této rovnici jsou všechny proměnné, které nejsou označeny horním indexem $i + 1$, považovány za probíhající v časovém kroku i ($i + \Delta t = t + \Delta t$). Dolní index j označuje plošný krok x ($j + \Delta x = x + \Delta x$).

Numerická stabilita řešení je zajištěna, pokud pro časový a polohový krok platí:

$$c \frac{\Delta t}{\Delta x} \leq 1 \quad (17)$$

kde pro desku

$$c = m \cdot y^{m-1} \quad (18)$$

Kromě řešení následných hloubek y_j^{i+1} řeší model KINFIL i ostatní proměnné procesu tvorby a transformace povrchového odtoku, především hydraulické rychlosti v_j^i :

$$v_j^i = \alpha_j \cdot (y_j^i)^{m_j-1} \quad (19)$$

dále třecí rychlosti $(v^*)^i_j$:

$$(v^*)^i_j = \sqrt{g \cdot Y_j \cdot y_j^i} \quad (20)$$

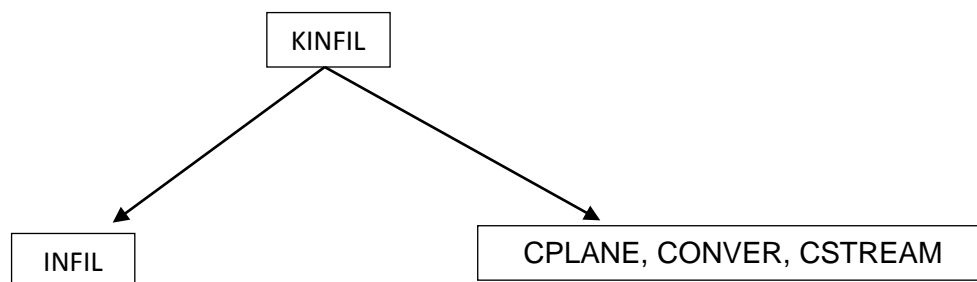
a tangenciální napětí τ_j^i :

$$\tau_j^i = \rho \cdot g \cdot Y_j \cdot y_j^i \quad (21)$$

kde α_j , m_j jsou hydraulické parametry, Y_j sklon pozemku (-), g gravitační zrychlení (m/s^2) a ρ hustota vody (kg/m^3).

4. Struktura modelu KINFIL

Model KINFIL se skládá ze dvou základních částí:

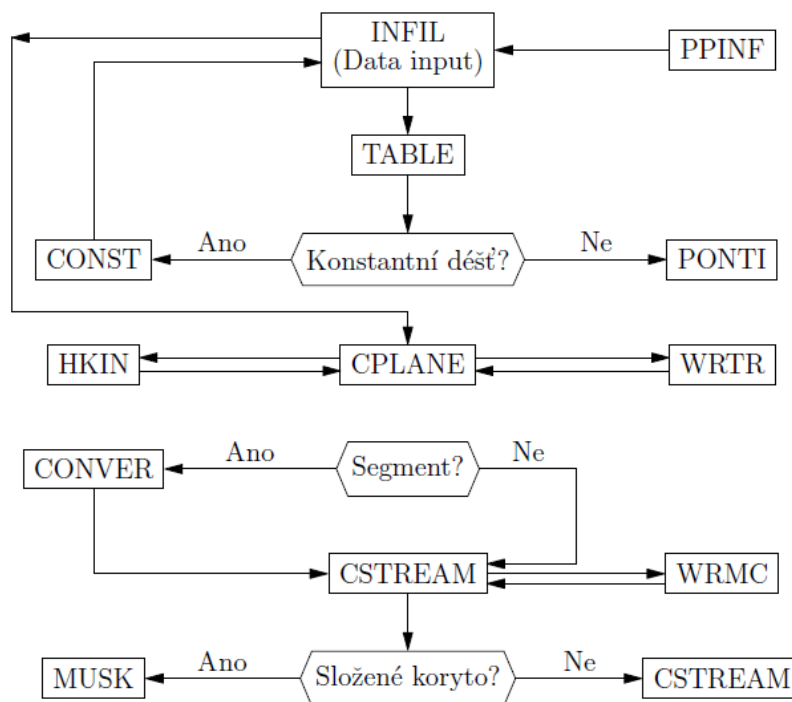


První část modelu je infiltrační submodel INFIL (celistvý modul), obsahující řešení infiltračního procesu, založeného na teorii Greena a Ampta, jak je výše popsáno. Součástí řešení je nalezení ekvivalence $CN = f(K_s, S_f)$. Dílčí podprogramy části INFIL jsou:

- PONTI: výpočet doby výtopy,
- CONST: infiltrace z deště o konstantní intenzitě,
- PPFIND: infiltrace z deště o proměnlivé intenzitě,
- TABLE: přiřazení parametru dle $CN = f(K_s, S_f)$.

Druhou částí je transformační submodel KINFIL, který řeší transformaci přímého odtoku. Procedura CPLANE je určena k simulaci odtoku po geometrizované kaskádě desek, procedura CONVER po segmentech, CSTREAM říční úseky. Numerické schéma řešení je explicitní, Laxe-Wendroffa. Pro řešení soustředěného odtoku po říčních úsecích s laterálními přítoky (CPLANE, CONVER) je možno alternativně řešit metodou MUSKINGUM-CUNGE, která je rovněž obsahem modelu KINFIL, jako samostatná procedura. Všechny tyto zmíněné procedury jsou vzájemně kompatibilní a tvoří model KINFIL. Přidružené pomocné procedury a funkce jsou:

- WRTR: pro tisk mezivýsledků CPLANE a CONVER
- WRMC: pro tisk mezivýsledků CSTREAM
- HKIN: pro řešení numerického schématu Lax-Wendroff



Popsaný model KINFIL je typickým příkladem modelu simulačního typu, popisující významný srážko-odtokový případ a nikoliv empirický model erozního procesu typu univerzální rovnice ztráty pudy (Wischmeier, Smith, 1978; Váška, 2000; Janeček et al., 2002).

Potřeba vstupních dat modelu je v následujících tabulkách, rozlišujících model KINFIL na část INFIL a část KIN. Popis parametru i proměnných je stručně v tabulkách uveden. Vzory vstupních dat jsou poskytnuty v přílohách A a B, jednotlivé symboly v nich značí

- část INFIL:

SUBOPT1, 2, 3, 4	logické proměnné (0 nebo 1)
QO	počáteční průtok (m^3/s)
KT	koefficient nasycené hydraulické vodivosti (mm/hod)
SO	koefficient sorptivity ($\text{mm}/\text{hod}^{0.5}$)
P	celková výška srážky (mm)
TD	doba trvání deště (hod)
CN	číslo odtokové křivky (není nutné)
N	počet pořadnic deště (-)
JJ	počet pořadnic hydrogramu (-)
DELT	délka časového kroku (hod)
RAIN (I)	výšky deště v časových krocích – pořadnic hyetogramu (mm)
FLAG	návěští zda pokračovat (1), nebo zastavit (2)

- část KIN:

NPL	počet soustav desek/segmentu (-)
PP	počet desek/segmentu v kaskádě jedné soustavy (-)
SLOPE	sklon svahu (-)
LENGTH	délka svahu (m)
WIDTH	šířka svahu (m)
OBST	překážka na svahu – relativně v desetinném zlomku záběru (-)
MAN	Manningova drsnost n (-)
FRIC	hydraulická turbulence (0.6)

TYPF	typ proudění (1.67)
DELT	délka časového kroku KIN (s)
TDELT	celková doba trvání případu (s)
NN	počet poradnic efektivního deště (-)
EFF RAIN (I)	pořadnice efektivního deště z části INFIL (mm)
FLAG	návěští zda pokračovat (1), nebo zastavit (0)

Posledními dvěma přílohami jsou ukázky výpočtu obou částí modelu KINFIL (INFIL a KIN).

5. Literatura

- Beven K. J. (1986): Runoff production and flood frequency in catchments of order n: An alternative approach. In: Gupta, V. K. (ed.): Scale Problems in Hydrology. D. Riedel Publishing Comp., s. 107–131.
- Beven K. J. (2006): Rainfall–Runoff Modelling. The Primer. John Willey & Sons, Chichester.
- Janeček M., a kol. (2002): Ochrana zemědělské půdy před erozí. ISV nakladatelství, Praha, ISBN 85866-85-8, 201 s.
- Cunge J. A. (1969): On the subject of a flood propagation computation method (Muskingum method). Journal of Hydraulic Research, 7(2): 205–230.
- Kovář P. (1992): Možnosti stanovení návrhových průtoků na malých povodích modelem KINFIL. Vodohospodářský časopis, 40(2): 197–220.
- Kovář P. (2000): Využití hydrologických modelů pro určování maximálních průtoků na malých povodích. SIC CZU, Praha.
- Lax P. D., Wendroff B. (1960): System of Conservation Laws. Communication on Pure and Applied Mathematics, 13(2): 217–237.
- Mein R. G., Larson C. L. (1973): Modelling infiltration during a steady rain. Water Resources Research, 9(2): 384–394.
- Morel-Seytoux H. J. (1976): Derivation of equations for rainfall infiltration. Journal of Hydrology, 31: 203–219.
- Morel-Seytoux H. J. (1982): Analytical results for prediction of variable rainfall infiltration. Journal of Hydrology, 59: 209–230.
- Morel-Seytoux H. J., VERDIN J. P. (1981): Extension of the Soil Conservation Service Rainfall–runoff methodology for ungauged watersheds. Colorado State University.
- Morgan, R. P. C., Nearing, M. A. (2011): Handbook of Erosion Modelling. Willey & Blackwell, ISBN 978-1-4051-9010-7, 401 s.
- Overton D. E., Meadows M. E. (1976): Stormwater Modelling. Academic Press New York.
- US SOIL CONSERVATION SERVICE (1972): National Engineering Handbook. Section 4, Hydrology. Washington, D. C.
- US SOIL CONSERVATION SERVICE (1986): Urban Hydrology for Small Watersheds. Technical Release 55 (updated), USA.
- Stephenson D., Meadows M. E. (1986): Kinematic Hydrology and Modelling. Elsevier.
- VÁŠKA, J. (2000): Hydromeliorace. ČKAIT, ISBN 80 86426-01-7, 220 s.
- Wischmeier W. H., Smith D. D. (1978): Predicting Rainfall Erosion Losses – A Guide Book to Conservation Planning. Agrarian Handbook No. 537, US Dept. of Agriculture, Washington.

A Vzor vstupních dat – část INFIL

```
TREBSIN, Locality 6, 23.7.2008 AREA=30m2, DRY, DT=1min, maize (Info-hlavička)
0 0 0 0 (SUBPT1, 2, 3, 4)
0 (QO)
7.84 15.21 16.10 0.250 80.00 (KT, SO, P, TD, CN)
15 20 0.0167 (N, JJ, DELT)
0.98 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
1.08 1.08 1.08 1.08 1.08 (EFF RAIN (I), I = 1,NN)
1 (FLAG)
TREBSIN, Locality , 23.7.2008 AREA=30m2, WET, DT=1min, maize (dtto)
0 0 0 0
0
7.84 8.21 16.17 0.250 80.00
16 20 0.0167
0.05 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08 1.08
1.08 1.08 1.08 1.08 1.08
0
```

B Vzor vstupních dat – část KIN

```
0.0 (QO)
TREBSIN, Locality 6,23.7.2008,DT=1min,AREA=30m2,DRY,maize (Info-hlavička)
1 (NPL)
1 (PP)
0.128 10.0 3.0 0.0 0.10 0.6 1.67 (SLOPE, LENGHT, WIDTH, OBST,
MAN, FRIC, TYPF)
60.0 1200.0 (DELT, TDELT)
16 (NN)
0.000 0.000 0.016 0.147 0.296 0.384 0.444 0.487 0.2117 0.549
0.571 0.591 0.607 0.621 0.634 0.645 (EFF RAIN (I), I = 1,NN)
1 (FLAG)
TREBSIN, Locality 6,23.7.2008,DT=1min,AREA=30m2,WET,maize (dtto)
1
1
0.128 10.0 3.0 0.0 0.10 0.6 1.67
60.0 1200.0
16
0.000 0.176 0.421 0.554 0.619 0.660 0.688 0.710 0.727 0.740
0.752 0.762 0.770 0.777 0.784 0.790
0
```

C Výstup z části INFIL

T H E I N F I L T R A T I O N M O D E L

```
*****
THE KW-INFILTRATION MODEL COMPUTES NET RAINFALL FROM GROSS ONE USING
MEIN - LARSON AND MOREL - SEYTOUX INFILTRATION FORMULAE
FLOOD HYDROGRAPH CAN BE COMPUTED USING UH CONVOLUTION
```

```
IF SUBOPT1=1, VARIABLE RAINFALL RATES IS USED
IF SUBPT1=0, CONSTANT RAINFALL IS USED
IF SUBPT2=0, USER INPUTS KT AND SFFC
IF SUBPT2=1, KT AND SFFC ARE COMPUTED FROM CN
IF SUBPT3=1, MEASURED DISCHARGES SHOULD BE READ
IF SUBPT4=0, EROSION PROGRAM IS IMPLEMENTED
```

NAME OF CATCHMENT:

```
TREBSIN, Locality 6 23.7.2008 AREA=30 m2 DRY DT=1min maize
*****
SUBOPT1= 0 SUBOPT2= 0 SUBOPT3= 0
SUBOPT4= 0
```

```
HYDRAULIC CONDUCTIVITY, KT = 7.840 MM/HR
SORPTIVITY, SO = 15.210 MM/HR**0.5
STORAGE SUCTION FACTOR, SFFC = 14.754 MM
TOTAL PRECIP, P = 16.100 MM
DURATION TIME,TD = .250 HR
```

OUTPUT OF SUBROUTINE CONST, CONSTANT RAINFALL

BY INFILTRATION APPROACH

PONDING TIME TP= .032 HR

T(HR)	W(MM)	DELW(MM)	IR(MM/HR)	R(MM/HR)	RE(MM/HR)	RER(MM/HR)
.032	2.045	2.045				
.017	.000	.000	.000	.980	.000	.000
.033	2.149	.104	63.442	1.080	.016	.016
.050	3.078	.928	55.578	1.080	.147	.147
.067	3.857	.779	46.650	1.080	.296	.296
.083	4.548	.691	41.401	1.080	.384	.384
.100	5.180	.632	37.838	1.080	.444	.444
.117	5.768	.588	35.215	1.080	.487	.487
.134	6.322	.554	33.179	1.080	.521	.521
.150	6.849	.527	31.539	1.080	.549	.549
.167	7.353	.504	30.182	1.080	.571	.571
.184	7.838	.485	29.034	1.080	.591	.591
.200	8.306	.468	28.047	1.080	.607	.607
.217	8.760	.454	27.186	1.080	.621	.621
.234	9.201	.441	26.426	1.080	.634	.634
.250	9.631	.430	25.750	1.080	.645	.645
.267	.000	.000	.000	.000	.000	.000

MASS BALANCE CHECK

EXCESS PRECIP= 6.469 MM
 CUMULATIVE INFILTRATION= 9.631 MM
 RETENTION= .000 MM
 TOTAL PRECIP= 16.100 MM

NAME OF CATCHMENT:

TREBSIN, Locality 6 23.7.2008 AREA=30 m2 WET DT=1min maize

 SUBOPT1= 0 SUBOPT2= 0 SUBOPT3= 0
 SUBOPT4= 0

HYDRAULIC CONDUCTIVITY, KT = 7.840 MM/HR
 SORPTIVITY, SO = 8.210 MM/HR**0.5
 STORAGE SUCTION FACTOR, SFFC = 4.299 MM
 TOTAL PRECIP, P = 16.170 MM
 DURATION TIME,TD = .250 HR

OUTPUT OF SUBROUTINE CONST, CONSTANT RAINFALL
 BY INFILTRATION APPROACH

PONDING TIME TP= .009 HR

T(HR)	W(MM)	DELW(MM)	IR(MM/HR)	R(MM/HR)	RE(MM/HR)	RER(MM/HR)
.009	.593	.593				
.017	1.001	.408	54.156	.050	.176	.176
.033	1.660	.659	39.449	1.080	.421	.421
.050	2.186	.527	31.527	1.080	.554	.554
.067	2.648	.461	27.628	1.080	.619	.619
.083	3.068	.421	25.189	1.080	.660	.660
.100	3.460	.392	23.477	1.080	.688	.688
.117	3.831	.371	22.189	1.080	.710	.710
.134	4.185	.354	21.176	1.080	.727	.727
.150	4.524	.340	20.351	1.080	.740	.740
.167	4.853	.328	19.662	1.080	.752	.752
.184	5.171	.319	19.076	1.080	.762	.762
.200	5.481	.310	18.569	1.080	.770	.770
.217	5.784	.303	18.126	1.080	.777	.777
.234	6.080	.296	17.733	1.080	.784	.784
.250	6.371	.290	17.382	1.080	.790	.790
.267	.000	.000	.000	.000	.000	.000

MASS BALANCE CHECK

EXCESS PRECIP= 9.799 MM
 CUMULATIVE INFILTRATION= 6.371 MM
 RETENTION= .000 MM
 TOTAL PRECIP= 16.170 MM

D Výstup z části KIN

MODEL KINFIL: HYDRAULIC VARIABLES,
FINAL HYDROGRAPH

LEGEND:

H(J,I) ... Depth of water (m)
VE(J,I) ... Velocity of water flow (m/s)
TAU(J,I) ... Shear stress (Pa)
VSTAR(J,I) ... Shear velocity (m/s)

NAME OF CATCHMENT: TREBSIN, Locality 6,30.7.2008,DT=1min,AREA=30m2,DRY,maize

DESCRIPTION OF PLANES:

SLOPES(-) LENGTHS(M) WIDTHS(M) MAN.ROUGHNESS M-FLOWTYPE
.128 10.000 3.000 .1000 1.6700

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE 1 AT TIME 2
J= 1 T= 2
H(J,I): .0000 .0000 .0000 .0000 .0000 .0000
VE(J,I): .0000 .0007 .0007 .0007 .0007 .0007
TAU(J,I): .0000 .0035 .0035 .0035 .0035 .0035
VSTR(J,I): .0000 .0019 .0019 .0019 .0019 .0019

1 2 .03 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE 1 AT TIME 3
J= 1 T= 3
H(J,I): .0000 .0001 .0001 .0001 .0001 .0001
VE(J,I): .0000 .0055 .0055 .0055 .0055 .0055
TAU(J,I): .0000 .0795 .0798 .0798 .0798 .0798
VSTR(J,I): .0000 .0089 .0089 .0089 .0089 .0089

2 3 .05 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE 1 AT TIME 4
J= 1 T= 4
H(J,I): .0000 .0002 .0003 .0003 .0003 .0003
VE(J,I): .0000 .0138 .0143 .0143 .0143 .0143
TAU(J,I): .0000 .3125 .3307 .3310 .3310 .3310
VSTR(J,I): .0000 .0177 .0182 .0182 .0182 .0182

3 4 .07 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE 1 AT TIME 5
J= 1 T= 5
H(J,I): .0000 .0004 .0006 .0006 .0006 .0006
VE(J,I): .0000 .0203 .0237 .0244 .0244 .0244
TAU(J,I): .0000 .5559 .7041 .7325 .7345 .7346
VSTR(J,I): .0000 .0236 .0265 .0271 .0271 .0271

4 5 .08 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE 1 AT TIME 6
J= 1 T= 6

H(J,I):	.0000	.0005	.0008	.0009	.0010	.0010
VE(J,I):	.0000	.0232	.0295	.0328	.0342	.0345
TAU(J,I):	.0000	.6787	.9741	1.1433	1.2152	1.2298
VSTR(J,I):	.0000	.0261	.0312	.0338	.0349	.0351

5 6 .10 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE	1	AT TIME	7			
J= 1	T=	7				
H(J,I):	.0000	.0006	.0009	.0011	.0012	.0013
VE(J,I):	.0000	.0246	.0320	.0370	.0405	.0424
TAU(J,I):	.0000	.7424	1.1011	1.3654	1.5615	1.6768
VSTR(J,I):	.0000	.0272	.0332	.0370	.0395	.0409

6 7 .12 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE	1	AT TIME	8			
J= 1	T=	8				
H(J,I):	.0000	.0006	.0009	.0012	.0014	.0015
VE(J,I):	.0000	.0255	.0334	.0390	.0433	.0466
TAU(J,I):	.0000	.7845	1.1744	1.4777	1.7284	1.9263
VSTR(J,I):	.0000	.0280	.0343	.0384	.0416	.0439

7 8 .13 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE	1	AT TIME	9			
J= 1	T=	9				
H(J,I):	.0000	.0006	.0010	.0012	.0015	.0016
VE(J,I):	.0000	.0262	.0344	.0402	.0449	.0487
TAU(J,I):	.0000	.8147	1.2246	1.5488	1.8242	2.0603
VSTR(J,I):	.0000	.0285	.0350	.0394	.0427	.0454

8 9 .15 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE	1	AT TIME	10			
J= 1	T=	10				
H(J,I):	.0000	.0007	.0010	.0013	.0015	.0017
VE(J,I):	.0000	.0267	.0351	.0411	.0460	.0501
TAU(J,I):	.0000	.8382	1.2626	1.6006	1.8907	2.1459
VSTR(J,I):	.0000	.0290	.0355	.0400	.0435	.0463

9 10 .17 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE	1	AT TIME	11			
J= 1	T=	11				
H(J,I):	.0000	.0007	.0010	.0013	.0015	.0018
VE(J,I):	.0000	.0271	.0357	.0418	.0468	.0510
TAU(J,I):	.0000	.8572	1.2928	1.6411	1.9415	2.2083
VSTR(J,I):	.0000	.0293	.0360	.0405	.0441	.0470

10 11 .18 .000

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076 .0380 .1900 1.6700

PLANE	1	AT TIME	12			
-------	---	---------	----	--	--	--

```

J= 1  T= 12
H(J,I): .0000 .0007 .0010 .0013 .0016 .0018
VE(J,I): .0000 .0274 .0361 .0424 .0475 .0518
TAU(J,I): .0000 .8727 1.3174 1.6739 1.9822 2.2573
VSTR(J,I): .0000 .0295 .0363 .0409 .0445 .0475

```

```
-----
11 12 .20 .000
-----
```

```
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
```

```
-----
.0076 .0380 .1900 1.6700
-----
```

```

PLANE 1 AT TIME 13
J= 1  T= 13
H(J,I): .0000 .0007 .0011 .0014 .0016 .0018
VE(J,I): .0000 .0277 .0365 .0429 .0480 .0524
TAU(J,I): .0000 .8861 1.3383 1.7014 2.0160 2.2976
VSTR(J,I): .0000 .0298 .0366 .0412 .0449 .0479

```

```
-----
12 13 .22 .000
-----
```

```
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
```

```
-----
.0076 .0380 .1900 1.6700
-----
```

```

PLANE 1 AT TIME 14
J= 1  T= 14
H(J,I): .0000 .0007 .0011 .0014 .0016 .0019
VE(J,I): .0000 .0279 .0368 .0432 .0485 .0529
TAU(J,I): .0000 .8974 1.3560 1.7248 2.0446 2.3314
VSTR(J,I): .0000 .0300 .0368 .0415 .0452 .0483

```

```
-----
13 14 .23 .000
-----
```

```
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
```

```
-----
.0076 .0380 .1900 1.6700
-----
```

```

PLANE 1 AT TIME 15
J= 1  T= 15
H(J,I): .0000 .0007 .0011 .0014 .0016 .0019
VE(J,I): .0000 .0281 .0371 .0436 .0489 .0534
TAU(J,I): .0000 .9075 1.3717 1.7453 2.0696 2.3607
VSTR(J,I): .0000 .0301 .0370 .0418 .0455 .0486

```

```
-----
14 15 .25 .000
-----
```

```
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
```

```
-----
.0076 .0380 .1900 1.6700
-----
```

```

PLANE 1 AT TIME 16
J= 1  T= 16
H(J,I): .0000 .0007 .0011 .0014 .0017 .0019
VE(J,I): .0000 .0283 .0374 .0439 .0492 .0538
TAU(J,I): .0000 .9167 1.3859 1.7637 2.0919 2.3868
VSTR(J,I): .0000 .0303 .0372 .0420 .0457 .0489

```

```
-----
15 16 .27 .000
-----
```

```
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
```

```
-----
.0076 .0380 .1900 1.6700
-----
```

```

PLANE 1 AT TIME 17
J= 1  T= 17
H(J,I): .0000 .0004 .0007 .0009 .0012 .0014
VE(J,I): .0000 .0176 .0265 .0334 .0391 .0441
TAU(J,I): .0000 .4520 .8309 1.1751 1.4863 1.7737
VSTR(J,I): .0000 .0213 .0288 .0343 .0386 .0421

```

```
-----
16 17 .28 .000
-----
```

```
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
```

```
-----
.0076 .0380 .1900 1.6700
-----
```

```

PLANE 1 AT TIME 18
J= 1 T= 18
  H(J,I): .0000 .0002 .0004 .0006 .0008 .0010
  VE(J,I): .0000 .0117 .0189 .0251 .0305 .0353
  TAU(J,I): .0000 .2466 .5030 .7648 1.0221 1.2761
VSTR(J,I): .0000 .0157 .0224 .0277 .0320 .0357

```

```
-----
17 18 .30 .000
-----
```

```
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
```

```
.0076 .0380 .1900 1.6700
```

```

PLANE 1 AT TIME 19
J= 1 T= 19
  H(J,I): .0000 .0001 .0003 .0004 .0006 .0007
  VE(J,I): .0000 .0085 .0143 .0195 .0242 .0287
  TAU(J,I): .0000 .1523 .3306 .5242 .7249 .9342
VSTR(J,I): .0000 .0123 .0182 .0229 .0269 .0306

```

```
-----
18 19 .32 .000
-----
```

```
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
```

```
.0076 .0380 .1900 1.6700
```

```

PLANE 1 AT TIME 20
J= 1 T= 20
  H(J,I): .0000 .0001 .0002 .0003 .0004 .0006
  VE(J,I): .0000 .0065 .0113 .0157 .0197 .0237
  TAU(J,I): .0000 .1028 .2326 .3789 .5353 .7039
VSTR(J,I): .0000 .0101 .0153 .0195 .0231 .0265

```

```
-----
19 20 .33 .000
-----
```

```
THE OVERLAND FLOW HYDROGRAPH:
```

```
ORDIN.NO. TIME(HOURS) DISCHARGE Q (L/S)
```

```

1 .033 .000
2 .050 .001
3 .067 .011
4 .083 .043
5 .100 .101
6 .117 .170
7 .133 .214
8 .150 .240
9 .167 .257
10 .183 .269
11 .200 .279
12 .217 .288
13 .233 .295
14 .250 .301
15 .267 .307
16 .283 .187
17 .300 .108
18 .317 .064
19 .333 .040

```

```
THE MASS BALANCE CHECK:
```

```
-----
TOTAL INFLOW DEPTH: 6.58 MM
TOTAL OUTFLOW DEPTH: 6.35 MM
-----
```

```
NAME OF CATCHMENT: TREBSIN, Locality 6,30.7.2008,DT=1min,AREA=30m2,WET,maize
```

```
*****
```

```
DESCRIPTION OF PLANES:
```

```
-----
SLOPES(-) LENGTHS(M) WIDTHS(M) MAN.ROUGHNESS M-FLOWTYPE
.128 10.000 3.000 .1000 1.6700
-----
```

```
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
```

```

.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 2
J= 1 T= 2
  H(J,I): .0000 .0001 .0001 .0001 .0001 .0001
  VE(J,I): .0000 .0097 .0098 .0098 .0098 .0098
  TAU(J,I): .0000 .1855 .1879 .1879 .1879 .1879
  VSTR(J,I): .0000 .0136 .0137 .0137 .0137 .0137
-----
      1          2          .03          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 3
J= 1 T= 3
  H(J,I): .0000 .0005 .0005 .0005 .0005 .0005
  VE(J,I): .0000 .0208 .0228 .0229 .0229 .0229
  TAU(J,I): .0000 .5783 .6627 .6693 .6695 .6695
  VSTR(J,I): .0000 .0240 .0257 .0259 .0259 .0259
-----
      2          3          .05          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 4
J= 1 T= 4
  H(J,I): .0000 .0006 .0009 .0010 .0010 .0011
  VE(J,I): .0000 .0260 .0327 .0354 .0361 .0362
  TAU(J,I): .0000 .8074 1.1354 1.2802 1.3182 1.3229
  VSTR(J,I): .0000 .0284 .0337 .0358 .0363 .0364
-----
      3          4          .07          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 5
J= 1 T= 5
  H(J,I): .0000 .0007 .0010 .0013 .0015 .0016
  VE(J,I): .0000 .0276 .0361 .0417 .0455 .0474
  TAU(J,I): .0000 .8818 1.3149 1.6350 1.8612 1.9801
  VSTR(J,I): .0000 .0297 .0363 .0404 .0431 .0445
-----
      4          5          .08          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 6
J= 1 T= 6
  H(J,I): .0000 .0007 .0011 .0014 .0016 .0018
  VE(J,I): .0000 .0284 .0373 .0437 .0487 .0526
  TAU(J,I): .0000 .9220 1.3855 1.7514 2.0596 2.3127
  VSTR(J,I): .0000 .0304 .0372 .0418 .0454 .0481
-----
      5          6          .10          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 7
J= 1 T= 7
  H(J,I): .0000 .0008 .0011 .0014 .0017 .0019
  VE(J,I): .0000 .0290 .0382 .0447 .0500 .0545
  TAU(J,I): .0000 .9491 1.4304 1.8141 2.1439 2.4347
  VSTR(J,I): .0000 .0308 .0378 .0426 .0463 .0493
-----
      6          7          .12          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----

```

```

-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 8
J= 1 T= 8
  H(J,I): .0000 .0008 .0012 .0015 .0018 .0020
  VE(J,I): .0000 .0294 .0387 .0455 .0509 .0555
  TAU(J,I): .0000 .9691 1.4626 1.8580 2.1996 2.5042
  VSTR(J,I): .0000 .0311 .0382 .0431 .0469 .0500
-----
      7          8          .13          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 9
J= 1 T= 9
  H(J,I): .0000 .0008 .0012 .0015 .0018 .0020
  VE(J,I): .0000 .0297 .0392 .0460 .0515 .0563
  TAU(J,I): .0000 .9842 1.4867 1.8905 2.2403 2.5536
  VSTR(J,I): .0000 .0314 .0386 .0435 .0473 .0505
-----
      8          9          .15          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 10
J= 1 T= 10
  H(J,I): .0000 .0008 .0012 .0015 .0018 .0021
  VE(J,I): .0000 .0299 .0395 .0464 .0520 .0568
  TAU(J,I): .0000 .9956 1.5050 1.9150 2.2708 2.5903
  VSTR(J,I): .0000 .0316 .0388 .0438 .0477 .0509
-----
      9          10         .17          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 11
J= 1 T= 11
  H(J,I): .0000 .0008 .0012 .0015 .0018 .0021
  VE(J,I): .0000 .0301 .0398 .0467 .0524 .0572
  TAU(J,I): .0000 1.0058 1.5208 1.9356 2.2960 2.6202
  VSTR(J,I): .0000 .0317 .0390 .0440 .0479 .0512
-----
     10         11         .18          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 12
J= 1 T= 12
  H(J,I): .0000 .0008 .0012 .0016 .0018 .0021
  VE(J,I): .0000 .0303 .0400 .0470 .0527 .0576
  TAU(J,I): .0000 1.0143 1.5341 1.9531 2.3174 2.6452
  VSTR(J,I): .0000 .0318 .0392 .0442 .0481 .0514
-----
     11         12         .20          .000
-----
LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):
-----
.0076      .0380      .1900      1.6700

PLANE 1 AT TIME 13
J= 1 T= 13
  H(J,I): .0000 .0008 .0012 .0016 .0019 .0021
  VE(J,I): .0000 .0304 .0402 .0472 .0530 .0579
  TAU(J,I): .0000 1.0212 1.5449 1.9674 2.3349 2.6658
  VSTR(J,I): .0000 .0320 .0393 .0444 .0483 .0516
-----
     12         13         .22          .000
-----

```


LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076	.0380	.1900	1.6700			

PLANE	1	AT TIME	14			
J= 1	T=	14				
H(J,I):	.0000	.0008	.0012	.0016	.0019	.0021
VE(J,I):	.0000	.0306	.0403	.0474	.0532	.0582
TAU(J,I):	.0000	1.0278	1.5549	1.9803	2.3505	2.6841
VSTR(J,I):	.0000	.0321	.0394	.0445	.0485	.0518

13	14	.23	.000			

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076	.0380	.1900	1.6700			

PLANE	1	AT TIME	15			
J= 1	T=	15				
H(J,I):	.0000	.0008	.0012	.0016	.0019	.0021
VE(J,I):	.0000	.0307	.0405	.0476	.0534	.0584
TAU(J,I):	.0000	1.0329	1.5631	1.9911	2.3636	2.6994
VSTR(J,I):	.0000	.0321	.0395	.0446	.0486	.0520

14	15	.25	.000			

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076	.0380	.1900	1.6700			

PLANE	1	AT TIME	16			
J= 1	T=	16				
H(J,I):	.0000	.0008	.0013	.0016	.0019	.0022
VE(J,I):	.0000	.0308	.0406	.0478	.0536	.0586
TAU(J,I):	.0000	1.0378	1.5706	2.0007	2.3753	2.7130
VSTR(J,I):	.0000	.0322	.0396	.0447	.0487	.0521

15	16	.27	.000			

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076	.0380	.1900	1.6700			

PLANE	1	AT TIME	17			
J= 1	T=	17				
H(J,I):	.0000	.0004	.0007	.0010	.0013	.0015
VE(J,I):	.0000	.0176	.0273	.0349	.0411	.0465
TAU(J,I):	.0000	.4511	.8675	1.2498	1.5985	1.9219
VSTR(J,I):	.0000	.0212	.0295	.0354	.0400	.0438

16	17	.28	.000			

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076	.0380	.1900	1.6700			

PLANE	1	AT TIME	18			
J= 1	T=	18				
H(J,I):	.0000	.0002	.0004	.0006	.0008	.0011
VE(J,I):	.0000	.0112	.0188	.0253	.0311	.0364
TAU(J,I):	.0000	.2282	.4960	.7766	1.0563	1.3329
VSTR(J,I):	.0000	.0151	.0223	.0279	.0325	.0365

17	18	.30	.000			

LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa):

.0076	.0380	.1900	1.6700			

PLANE	1	AT TIME	19			
J= 1	T=	19				
H(J,I):	.0000	.0001	.0003	.0004	.0006	.0008
VE(J,I):	.0000	.0079	.0138	.0192	.0242	.0289
TAU(J,I):	.0000	.1356	.3144	.5145	.7262	.9464
VSTR(J,I):	.0000	.0116	.0177	.0227	.0269	.0308

18	19	.32	.000			

LIMITS FOR CRITICAL SHEAR STRESS τ_{Ucr} (Pa):

	.0076	.0380	.1900	1.6700		
PLANE 1						
J= 1						
AT TIME						
T= 20						
H(J,I):	.0000	.0001	.0002	.0003	.0004	.0006
VE(J,I):	.0000	.0060	.0108	.0153	.0195	.0236
TAU(J,I):	.0000	.0899	.2168	.3641	.5252	.6974
VSTR(J,I):	.0000	.0095	.0147	.0191	.0229	.0264
	19	20	.33	.000		

THE OVERLAND FLOW HYDROGRAPH:

ORDIN.NO. TIME(HOURS) DISCHARGE Q (L/S)

1	.033	.004
2	.050	.037
3	.067	.114
4	.083	.224
5	.100	.291
6	.117	.317
7	.133	.332
8	.150	.343
9	.167	.351
10	.183	.358
11	.200	.364
12	.217	.369
13	.233	.373
14	.250	.377
15	.267	.380
16	.283	.214
17	.300	.116
18	.317	.065
19	.333	.039

THE MASS BALANCE CHECK:

TOTAL INFLOW DEPTH:	9.54	MM
TOTAL OUTFLOW DEPTH:	9.34	MM

6. Software - zdrojový program modelu KINFIL (FORTRAN)

INFIL – data TŘEBSÍN

```
$DEBUG
$NOTRUNCATE
PROGRAM EINFIL
C VERSION OF INFIL MODEL FOR EROSION
C - KBUK FES CULS PRAGUE, 2014
C NAME OF FILE EINFIL.FOR
COMMON/A1/DELT,QU,N,P,TD,CN,II
COMMON/A2/KT,TP,RP,WP,K
COMMON/A3/MO,DAY,YEAR,SFFC,S,SO
COMMON/A4/T(250),R(250),RE(250),RER(250)
COMMON/A5/AR(5)
COMMON/A6/SF,RETEN
COMMON/A7/CUMP(250)
COMMON/A8/AREA,L,Y,NN,NF
COMMON/A9/DELTA(250),CNM
COMMON/A10/DELP(250),QA(250),TM(250),QM(250)
COMMON/A11/SUBPT1,SUBPT2,SUBPT3,SUBPT4
COMMON/A12/TL
COMMON/A13/CNAME(20)
INTEGER QU,SUBPT1,SUBPT2,SUBPT3,SUBPT4,DAY,YEAR
REAL KT,L
CHARACTER*12 NDFILE
CHARACTER*8 FNAME$
OPEN(UNIT=5,FILE='treb4sim.dta',STATUS='OLD')
OPEN(UNIT=7,FILE='treb4sim.out',STATUS='UNKNOWN')
OPEN(UNIT=1,FILE='NDFILE',STATUS='UNKNOWN')
62 FORMAT(2X,56(1H*))
C
63 FORMAT(2X,'THE INFILTRATION MODEL '/')
64 FORMAT(2X,'THE KW-INFILTRATION MODEL COMPUTES NET RAINFALL FROM GR
*OSS ONE USING'/2X,'MEIN - LARSON AND MOREL - SEYTOUX INFILTRATION
*FORMULAE')
61 FORMAT(2X,'FLOOD HYDROGRAPH CAN BE COMPUTED USING UH CONVOLUTION')
WRITE(6,63)
WRITE(6,62)
WRITE(6,64)
WRITE(6,61)
WRITE(6,51)
WRITE(7,63)
WRITE(7,62)
WRITE(7,64)
WRITE(7,61)
WRITE(7,51)
51 FORMAT(//,2X,'IF SUBOPT1=1, VARIABLE RAINFALL RATES IS USED',/
*2X,'IF SUBPT1=0, CONSTANT RAINFALL IS USED',
*//,2X,'IF SUBPT2=0, USER INPUTS KT AND SFFC'/
*2X,'IF SUBPT2=1, KT AND SFFC ARE COMPUTED FROM CN'/
*2X,'IF SUBPT3=1, MEASURED DISCHARGES SHOULD BE READ'/
*2X,'IF SUBPT4=0, EROSION PROGRAM IS IMPLEMENTED'//)
C
76 FORMAT(20A4)
100 READ(5,76)CNAME
C
65 FORMAT(2X,'NAME OF CATCHMENT:'/2X,20A4/2X,60(1H*))
WRITE(6,65)CNAME
WRITE(7,65)CNAME
READ(5,38)SUBPT1,SUBPT2,SUBPT3,SUBPT4
38 FORMAT(I2,2X,I2,2X,I2,2X,I2)
READ(5,72)IDENT
72 FORMAT(I1)
WRITE(6,39)SUBPT1,SUBPT2,SUBPT3,SUBPT4
WRITE(7,39)SUBPT1,SUBPT2,SUBPT3,SUBPT4
39 FORMAT(2X,'SUBOPT1=',I2,3X,'SUBOPT2=',I2,3X,'SUBOPT3=',I2/
*2X,'SUBOPT4=',I2)
IF(SUBPT2.EQ.0)GO TO 40
READ(5,41)P,TD,CN,CNM
41 FORMAT(4F10.3)
WRITE(6,42)P,TD,CN
WRITE(7,42)P,TD,CN
42 FORMAT(2X,'STORM DEPTH P =',F10.3,2X,'MM'/2X,'STORM DURATION TD ='
*,F10.3,2X,'HR'/2X,'CURVE NUMBER CN=',F10.3,2X,'(-)')
```

```

C      READ '(A12)',NDFILE
      CALL TABLE
      GO TO 43
40 READ(5,2)KT,SO,P,TD,CN
   2 FORMAT(5F10.3)
      SFFC=(SO**2)/(2.0*KT)
      WRITE(6,15)KT,SO,SFFC,P,TD
      WRITE(7,15)KT,SO,SFFC,P,TD
15 FORMAT(/,2X,'HYDRAULIC CONDUCTIVITY, KT =',F8.3,1X,'MM/HR',/,2X,
* 'SORPTIVITY, SO =',F8.3,1X,'MM/HR**0.5',/2X,
* 'STORAGE SUCTION FACTOR, SFFC =',F8.3,1X,'MM',/,
*2X,'TOTAL PRECIP, P =',F8.3,1X,'MM',/,2X,'DURATION TIME,TD =',F8.3
* ,1X,'HR',)
      GO TO 8
43 READ(5,27)AREA,L,Y,RETEN
27 FORMAT(4F10.5)
      WRITE(6,28)AREA,L,Y
      WRITE(7,28)AREA,L,Y
28 FORMAT(2X,'AREA=',F10.5,2X,'KM2',
*/,2X,'LENGTH TO DIVIDE=',F10.2,2X,'M'/2X,
* 'AVG CATCHMENT SLOPE=',F10.2,2X,'PERCENT')
   8 READ(5,9)N,II,DELT
   9 FORMAT(I3,2X,I3,2X,F10.5)
c     WRITE(6,20)N,II,DELT
c     WRITE(7,20)N,II,DELT
20 FORMAT(/,2X,'N=',I3,5X,'II =',I3,5X,'DELT =',F10.5/)
C
C     WRITE(6,49)
C     WRITE(7,49)
49 FORMAT(2X,'N IS NUMBER OF TIME STEPS IN USERS STORM'/
*2X,'II IS NUMBER OF TIME STEPS IN REQUIRED HYDROGRAPH' /
*2X,'DELT IS LENGHT OF TIME STEP IN HR' /
*2X,'INPUT RAIN IS IN MM/HR',/)
C
      IF(IDENT.NE.1)GOTO 33
C
      READ(5,11)DAY,MO,YEAR
11 FORMAT(I2,2X,I2,2X,I2)
      READ(5,16)(AR(I),I=1,5)
16 FORMAT(5F8.0)
C
      WRITE(6,13)DAY,MO,YEAR
      WRITE(7,13)DAY,MO,YEAR
13 FORMAT(2X,'DATE OF EVENT:',I2,'/',I2,'/','19',I2,/)
14 FORMAT(2X,'ANTECEDENT DAILY RAIN AR(I):'/2X,5F7.1/)
      WRITE(6,14)(AR(I),I=1,5)
      WRITE(7,14)(AR(I),I=1,5)
      READ(5,31)QM(1)
31 FORMAT(F10.3)
      WRITE(6,48)QM(1)
      WRITE(7,48)QM(1)
48 FORMAT(2X,'INITIAL DISCHARGE OBSERVED: QM(1)=' ,F10.3,1X,'M3/S',/)
33 CONTINUE
C
      SCREEN CONTROL
C
      READ '(A12)',NDFILE
99 T(1)=DELT
      DO 22 I=2,II
22 T(I)=T(I-1)+DELT
      READ(5,10)(R(I),I=1,N)
10 FORMAT(10F8.0)
   3 FORMAT(2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/,
* 2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/,
* /2X,10F7.2,/,2X,10F7.2,/,2X,10F7.2,/)
32 FORMAT(2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/,
* 2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/,
* /2X,10F6.3,/,2X,10F6.3,/,2X,10F6.3,/)
52 CONTINUE
C
      WRITE(6,36)
C
      WRITE(7,36)
36 FORMAT(2X,'TIME STEPS, HOURS')
C
      WRITE(6,3)(T(I),I=1,II)
45 CONTINUE
C
      WRITE(6,23)
C
      WRITE(7,3)(T(I),I=1,II)
C
      WRITE(7,23)
C
23 FORMAT(2X,'RAINFALL INTENSITIES, MM/HR')
C
      WRITE(6,3)(R(I),I=1,N)

```

```

C      WRITE(7,3)(R(I),I=1,N)
C      COMPUTE CUMULATIVE STEP PRECIP, CUMP
      DO 24 I=1,N
        IF(I.EQ.1)GO TO 25
        CUMP(I)=CUMP(I-1)+R(I)*(T(I)-T(I-1))
        GO TO 24
      25 CUMP(I)=R(I)*T(I)
      24 CONTINUE
C      WRITE(6,26)
C      WRITE(7,26)
      26 FORMAT(2X,'STEPS OF CUMULATIVE PRECIP (MM)')
C      WRITE(6,3)(CUMP(I),I=1,N)
C      WRITE(7,3)(CUMP(I),I=1,N)
        IF(SUBPT3.EQ.0)GO TO 80
        READ(5,56)(QM(I),I=1,II)
      56 FORMAT(10F8.0)
        WRITE(6,57)
        WRITE(7,57)
      57 FORMAT(2X,'THE OBSERVED DISCHARGE VALUES IN M3/S',/)
C      IF(IDENT.EQ.0)GO TO 74
C      DO 75 I=1,II
C      75 QM(I)=QM(I)*0.001
      74 CONTINUE
        WRITE(6,32)(QM(I),I=1,II)
        WRITE(7,32)(QM(I),I=1,II)
      80 CONTINUE
      5 READ(5,73)NEXT
      73 FORMAT(I1)
        SF=SF*FC
C      SCREEN CONTROL
C      READ '(A12)',NDFILE
        IF(SUBPT1.EQ.1)GO TO 30      !
        CALL CONST
        GO TO 66
      30 CONTINUE
        CALL PONTI
C      READ '(A12)',NDFILE
        CALL PPIF
      66 CONTINUE
        PRINT *, '**** END OF INFILTRATION PART OF MODEL ****'
        IF(SUBPT4.EQ.0)GO TO 6
        PRINT *, '**** BEGINING OF ROUTING PART OF MODEL ****'
        SRER=0.0
        DO 7 I=1,N
          7 SRER=SRER+RER(I)
          IF(SRER.LE.0.0)GO TO 6
      12 CALL UH
C      SCREEN CONTROL
C      READ '(A12)',NDFILE
        CALL ROUTE
C      SCREEN CONTROL
C      READ '(A12)',NDFILE
        IF(IDENT.EQ.1)CALL GODFI
C      SCREEN CONTROL
        READ '(A12)',NDFILE
        write(*,'(A)')' NAME OF EVENT DATA:'
        read(*,'(A)')FNAME$
        open(unit=8,file=FNAME$)
C      write(8,'(2x,i3,3f8.3)')(i,r(i),rer(i),qa(i),i=1,ii)
        write(8,'(2x,i3,f8.3)')(i,qa(i),i=1,ii)
        DO 4 I=1,II
          4 QA(I)=0.0
        close(8)
      6 CONTINUE
        IF(NEXT.NE.0) GO TO 100
      70 STOP
      END
      SUBROUTINE PONTI
      COMMON/A1/DELT,QU,N,P,TD,CN,II
      COMMON/A2/KT,TP,RP,WP,K
      COMMON/A4/T(250),R(250),RE(250),RER(250)
      COMMON/A6/SF,RETN
      DIMENSION PT(250)
      REAL KT
C
C      THIS SUBROUTINE CALCULATES PONDING TIME FOR A VARIABLE RAINFALL
C      INTENSITY EVENT

```

```

c      WRITE(6,33)
c      WRITE(7,33)
33  FORMAT(/2X,60(1H*),/)
      WRITE(6,27)
      WRITE(7,27)
27  FORMAT(/,2X,'OUTPUT OF SUBROUTINE PONTI',/)
      I=0
10  I=I+1
      IF(I.GT.N)GOTO 18
      IF(R(I).LE.KT)GOTO 10
      IF(I.EQ.1)GOTO 11
      II=I-1
      SUMP=0.
      DO 12 J=1,II
      IF(J.EQ.1)GOTO 14
      SUMP=SUMP+R(J)*(T(J)-T(J-1))
      GOTO 12
14  SUMP=SUMP+R(J)+T(J)
12  CONTINUE
      PT(I)=T(I-1)+(1./R(I))*((SF/((R(I)/KT)-1.))-SUMP)
C      TEST COMPUTED PONDING TIME AGAINST PREVIOUS TIME STEP
      IF(PT(I)-T(I-1))13,13,17
13  TP=T(I-1)
      RP=R(I)
      GOTO 23
C      TEST COMPUTED PONDING TIME AGAINST TIME STEP OF CONSIDERATION
17  IF(PT(I)-T(I))15,15,10
11  PT(I)=(1./R(I))*(SF/((R(I)/KT)-1.))
      IF(PT(I).GT.T(I))GO TO 10
15  TP=PT(I)
      RP=R(I)
23  K=0
      WP=0.0
      DO 20 J=1,N
      IF(T(J).GT.TP)GO TO 20
      IF(J.EQ.1)GO TO 21
      WP=WP+R(J)*(T(J)-T(J-1))
      GO TO 22
21  WP=WP+R(J)*T(J)
22  K=K+1
20  CONTINUE
      IF(K.EQ.0)GO TO 25
      WP=WP+RP*(TP-T(K))
      GO TO 26
25  WP=RP*TP
26  WRITE(6,24)TP,RP,WP
      WRITE(7,24)TP,RP,WP
24  FORMAT(2X,'PONDING TIME=',F8.3,1X,'HR',5X,'PONDING RAINFALL=',
      *F8.3,2X,'MM/HR'/2X,'DEPTH OF RAIN INFILTRATED PREVIOUS TO PONDING=
      *',F8.3,1X,'MM')
      IF(K.GT.0)GO TO 28
      GO TO 16
28  WRITE(6,29)K,T(K)
      WRITE(7,29)K,T(K)
29  FORMAT(2X,'LAST FULL TIME STEP T(',I3,')=',F8.3,1X,'HR')
      GO TO 16
18  WRITE(6,19)
      WRITE(7,19)
19  FORMAT(2X,'PONDING NEVER OCCURS')
16  continue
      RETURN
      END
      SUBROUTINE CONST
      COMMON/A1/DELT,QU,N,P,TD,CN,II
      COMMON/A2/KT,TP,RP,WP,K
      COMMON/A4/T(250),R(250),RE(250),RER(250)
      COMMON/A6/SF,RETEN
      COMMON/A8/AREA,L,Y,NN,NF
      COMMON/A10/DELP(250),QA(250),TM(250),QM(250)
      DIMENSION W(250),DELW(250),IR(250)
      REAL KT,IR
C
C      THIS SUBROUTINE COMPUTES EXCESS RAINFALL BY INFILTRATION EQUATION
C      FOR A CONSTANT INTENSITY EVENT
C
c      WRITE(6,33)
c      WRITE(7,33)

```

```

33 FORMAT(/2X,60(1H*),/)
WRITE(6,21)
WRITE(7,21)
21 FORMAT(/,2X,'OUTPUT OF SUBROUTINE CONST, CONSTANT RAINFALL '/
*2X,'BY INFILTRATION APPROACH',)
CR=P/TD
SORP =SQRT(2.*KT*SF)
RSTAR=CR/KT
IF(RSTAR.LE.1.)GO TO 4
C COMPUTE MEIN AND LARSON PONDING TIME
TP=SF/(CR*(RSTAR-1.0))
Write(6,35)TP
Write(7,35)TP
35 format(2x,'PONDING TIME TP= ',2X,f8.3,2X,'HR')
IF(TP.GE.TD)GO TO 4
KK=0
DO 22 I=1,N
IF(T(I).GE.TP)GO TO 22
KK=KK+1
22 CONTINUE
M=KK+1
RATIO=RSTAR/(RSTAR-1.0)
WP=CR*TP
B=0.5*TP*(RATIO**3)
DO 20 I=M,N
W(I)=WP+SORP*RATIO*(SQRT(T(I)-TP+B)-SQRT(B))+KT*(T(I)-TP)
IF(I.EQ.M)GO TO 11
DELW(I)=W(I)-W(I-1)
IR(I)=DELW(I)/(T(I)-T(I-1))
GO TO 12
11 DELW(I)=W(I)-WP
IR(I)=DELW(I)/(T(I)-TP)
12 CONTINUE
IF(CR-IR(I))13,13,14
13 IR(I)=CR
RE(I)=0.0
IF(I.EQ.M)GO TO 15
DELW(I)=CR*(T(I)-T(I-1))
W(I)=W(I-1)+DELW(I)
GO TO 20
15 DELW(I)=CR*(T(I)-TP)
W(I)=W(I-1)+DELW(I)
GO TO 20
14 RE(I)=CR-IR(I)
20 CONTINUE
C SUBTRACT RETENTION
RET=RETEN
DO 27 I=M,N
IF(I.EQ.M)GO TO 23
PS=RE(I)*(T(I)-T(I-1))
IF(RET-PS)26,25,25
26 RER(I)=(PS-RET)/(T(I)-T(I-1))
RET=0.0
GO TO 27
23 PS=RE(I)*(T(I)-TP)
IF(RET-PS)24,25,25
24 RER(I)=(PS-RET)/(T(I)-TP)
RET=0.0
GO TO 27
25 RER(I)=0.0
RET=RET-PS
27 CONTINUE
5 IFLAG=0
DO 28 I=M,N
IF(RER(I).EQ.0.0.AND.IFLAG.EQ.0)GO TO 28
IFLAG=IFLAG+1
DELT(IFLAG)=RER(I)*DELT
TM(IFLAG)=T(I)
28 CONTINUE
NK=N+1 !
DO 36 I=1,NK
RE(I)=RE(I)*DELT
RER(I)=RER(I)*DELT
36 CONTINUE
NIN=INT(TD/DELT)+1
c DO 1 I=NIN,N
c R(I)=0.0

```

```

c      RE(I)=0.0
c      RER(I)=0.0
1  CONTINUE
C      READ  '(A12)',NDFILE
      NF=IFLAG
      WRITE(6,17)
      WRITE(7,17)
17  FORMAT(5X,'T(HR)',6X,'W(MM)',3X,'DELW(MM)',1X,'IR(MM/HR)',2X,'R(MM
* /HR)',5X,'RE(MM/HR)',2X,'RER(MM/HR)',/)
      WRITE(6,18)TP,WP,WP
      WRITE(7,18)TP,WP,WP
18  FORMAT(1X,3F10.3)
      DO 16 I=1,NK
      WRITE(6,19)T(I),W(I),DELW(I),IR(I),R(I),RE(I),RER(I)
      WRITE(7,19)T(I),W(I),DELW(I),IR(I),R(I),RE(I),RER(I)
19  FORMAT(1X,7F10.3)
16  CONTINUE
C      READ  '(A12)',NDFILE
C      CHECK MASS BALANCE
      WRITE(6,7)
      WRITE(7,7)
7  FORMAT(/,2X,'MASS BALANCE CHECK',)
      PECONS=P-W(N)
6  RET=RETEN-RET
      WRITE(6,10)PECONS,W(N),RET,P
      WRITE(7,10)PECONS,W(N),RET,P
10  FORMAT(2X,'EXCESS PRECIP=',F8.3,2X,'MM',/,2X,
* 'CUMULATIVE INFILTRATION=',F8.3,2X,'MM',/,
* 2X,'RETENTION=',F8.3,2X,'MM',/,
* 2X,'TOTAL PRECIP=',F8.3,2X,'MM'//)
      IF(NF.EQ.0)GO TO 4
      GO TO 2
4  WRITE(6,3)
      WRITE(7,3)
3  FORMAT(/,5X,'ALL RAINFALL INFILTRATES - NO RUNOFF IS PRODUCED',/)
C  SCREEN CONTROL
2  CONTINUE
C  READ  '(A12)',NDFILE
      RETURN
      END
      SUBROUTINE TABLE
      COMMON/A1/DELT,QU,N,P,TD,CN,II
      COMMON/A2/KT,TP,RP,WP,K
      COMMON/A3/MO,DAY,YEAR,SFFC,S,SO
      COMMON/A11/SUBPT1,SUBPT2,SUBPT3,SUBPT4
      INTEGER DAY,YEAR
      REAL KT
      WRITE(6,33)
      WRITE(7,33)
33  FORMAT(/2X,60(1H*),/)
      WRITE(6,28)
      WRITE(7,28)
28  FORMAT(/,2X,'OUTPUT OF SUBROUTINE TABLE',/)
      IF(CN.LE.75.)GO TO 11
      KT=(100.-CN)/12.4
      GO TO 12
11  IF(CN.LE.36.)GO TO 13
      KT=31.394-0.391*CN
      GO TO 12
13  KT=47.066-0.823*CN
12  CONTINUE
      IF(CN.LE.65.)GOTO 14
      SORP=(100.-CN)/2.512
      GO TO 15
14  SORP=30.251-0.146*CN
15  CONTINUE
      SFFC=(SORP**2)/(2.*KT)
      WRITE(6,19)KT,SFFC
      WRITE(7,19)KT,SFFC
19  FORMAT(2X,'HYDRAULIC CONDUCTIVITY KT=',F10.3,2X,'MM/HR',/,2X,'STOR
* AGE SUCTION FACTOR AT FIELD CAPACITY SFFC=',F10.3,2X,'MM',/)
      WRITE(6,33)
      WRITE(7,33)
      RETURN
      END
      SUBROUTINE PPINF
      COMMON/A1/DELT,QU,N,P,TD,CN,II

```



```

COMMON/A2/KT,TP,RP,WP,K
COMMON/A3/MO,DAY,YEAR,SFFC,S,SO
COMMON/A4/T(250),R(250),RE(250),RER(250)
COMMON/A6/SF,RETEN
COMMON/A8/AREA,L,Y,NN,NF
COMMON/A10/DELP(250),QA(250),TM(250),QM(250)
DIMENSION W(250),DELW(250),IR(250)
INTEGER DAY,YEAR
REAL IR,KT

C
C THIS SUBROUTINE COMPUTES POST-PONDING INFILTRATION FOR A VARIABLE
C INTENSITY RAINFALL EVENT
WRITE(6,33)
WRITE(7,33)
33 FORMAT(/2X,60(1H*),/)
WRITE(6,21)
WRITE(7,21)
21 FORMAT(/2X,'OUTPUT OF SUBROUTINE PPFINF, VARIABLE RAINFALL'/
*2X,'INFILTRATION APPROACH'/)
RSORP=SQRT(2.*KT*((SF+WP)**2)/SF)
RSTARP=RP/KT
B=0.5*((SF+WP)**2)/(KT*SF*((RSTARP-1.)**2))
M=K+1
DO 10 I=M,N
W(I)=WP+RSORP*(SQRT(T(I)-TP+B)-SQRT(B))+KT*(T(I)-TP)
IF(I.EQ.M)GO TO 11
DELW(I)=W(I)-W(I-1)
IR(I)=DELW(I)/(T(I)-T(I-1))
GO TO 12
11 DELW(I)=W(I)-WP
IR(I)=DELW(I)/(T(I)-TP)
12 CONTINUE
IF(R(I)-IR(I))13,13,14
13 IR(I)=R(I)
RE(I)=0.0
IF(I.EQ.M)GO TO 15
DELW(I)=R(I)*(T(I)-T(I-1))
W(I)=W(I-1)+DELW(I)
GO TO 10
15 DELW(I)=R(I)*(T(I)-TP)
W(I)=W(I-1)+DELW(I)
GO TO 10
14 RE(I)=R(I)-IR(I)
10 CONTINUE
C SUBTRACT RETENTION
RET=RETEN
DO 27 I=M,N
IF(I.EQ.M)GO TO 23
PS=RE(I)*(T(I)-T(I-1))
IF(RET-PS)26,25,25
26 RER(I)=(PS-RET)/(T(I)-T(I-1))
RET=0.0
GO TO 27
23 PS=RE(I)*(T(I)-TP)
IF(RET-PS)24,25,25
24 RER(I)=(PS-RET)/(T(I)-TP)
RET=0.0
GO TO 27
25 RER(I)=0.0
RET=RET-PS
27 CONTINUE
IFLAG=0
DO 28 I=M,N
IF(RER(I).EQ.0.0.AND.IFLAG.EQ.0)GO TO 28
IFLAG=IFLAG+1
DELP(IFLAG)=RER(I)*DELT
TM(IFLAG)=T(I)
28 CONTINUE
C READ '(A12)',NDFILE
NF=IFLAG
WRITE(6,9)
WRITE(7,9)
9 FORMAT(5X,'T(HR) = TIME'//,15X,'W(MM) = CUMULATIVE INFILTRATION'//,
*15X,'DELW(MM) = INCREMENTAL INFILTRATION'//,
*15X,'IR(MM/HR) = INFILTRATION RATE'//,
*15X,'R(MM/HR) = RAINFALL RATE'//,
*15X,'RE(MM/HR) = RAINFALL RATE AFTER INFILTR. SUBTRACTED'//,

```

```

*15X,'RER(MM/HR) = NET RAINFALL AFTER RETENTION SUBTRACTED' /)
WRITE(6,17)
WRITE(7,17)
17 FORMAT(5X,'T(HR) ',6X,'W(MM) ',3X,'DELW(MM) ',1X,'IR(MM/HR) ', 'R(MM/
*HR) ',5X,'RE(MM/HR) ',2X,'RER(MM/HR) ', /)
WRITE(6,18)TP,WP,WP
WRITE(7,18)TP,WP,WP
18 FORMAT(1X,3F10.3)
DO 16 I=M,N
WRITE(6,19)T(I),W(I),DELW(I),IR(I),R(I),RE(I),RER(I)
WRITE(7,19)T(I),W(I),DELW(I),IR(I),R(I),RE(I),RER(I)
19 FORMAT(1X,7F10.3)
16 CONTINUE
C READ '(A12)',NDFILE
C CHECK MASS BALANCE
WRITE(6,7)
WRITE(7,7)
7 FORMAT(/,2X,'MASS BALANCE CHECK',/)
PE=0.0
IF(NF.EQ.0)GO TO 6
DO 8 I=1,NF
8 PE=PE+DELP(I)
6 RET=RETEN-RET
WRITE(6,20)PE,W(N),RET,P
WRITE(7,20)PE,W(N),RET,P
20 FORMAT(2X,'EXCESS PRECIP=',F8.3,2X,'MM',/,
*2X,'CUMULATIVE INFILTRATION=',F8.3,2X,'MM',/,
*2X,'RETENTION=',F8.3,2X,'MM',/,
*2X,'TOTAL PRECIP=',F8.3,2X,'MM',/)
IF(NF.EQ.0)GO TO 5
GO TO 3
5 WRITE(6,4)
WRITE(7,4)
4 FORMAT(5X,'ALL RAINFALL INFILTRATES - NO RUNOFF IS PRODUCED',/)
C SCREEN CONTROL
3 CONTINUE
C READ '(A12)',NDFILE
RETURN
END
SUBROUTINE UH
COMMON/A1/DELT,QU,N,P,TD,CN,II
COMMON/A4/T(250),R(250),RE(250),RER(250)
COMMON/A8/AREA,L,Y,NN,NF
COMMON/A9/DELTA(250),CNM
COMMON/A10/DELP(250),QA(250),TM(250),QM(250)
COMMON/A12/TL
DIMENSION RATIOQ(20),RATIOQ(20),QQT(250)
REAL L
C
WRITE(6,33)
WRITE(7,33)
33 FORMAT(2X,60(1H*),/)
WRITE(6,20)
20 FORMAT(/,2X,'OUTPUT OF SUBROUTINE UH',/)
C
S=25.4*((1000./(CN-5.0))-10.)
TL=((3.28*L)**0.8)*(((0.04*S)+1.0)**0.7)/(1900.0*(Y**0.5))
5 TTP=(DELT/2.)+TL
D=0.25*TTP
IF(DELT.LE.D)GO TO 9
WRITE(6,6)DELT,D
WRITE(7,6)DELT,D
6 FORMAT(2X,'THE TIME STEP OF',F5.2,1X,
*HR IS GREATER THAN 0.25 TIME TO PEAK',/2X,' WHICH IS',F5.2,
*1X,'HR',2X,'SO THE RESULTING HYDROGRAPH MAY BE JAGGED',/)
9 CONTINUE
WRITE(6,24)TL,TTP
WRITE(7,24)TL,TTP
24 FORMAT(2X,'WATERSHED LAG TIME =',F8.3,'HR',/,2X,'TIME TO PEAK=',
*F8.3,'HR')
DO 10 I=1,20
IF(I.EQ.1)GO TO 11
RATIOQ(I)=RATIOQ(I-1)+0.05
GO TO 10
11 RATIOQ(I)=0.05
10 CONTINUE
RATIOQ(1)=.47

```

```

RATIOT(2)=.60
RATIOT(3)=.69
RATIOT(4)=.78
RATIOT(5)=.85
RATIOT(6)=.92
RATIOT(7)=.97
RATIOT(8)=1.02
RATIOT(9)=1.08
RATIOT(10)=1.16
RATIOT(11)=1.24
RATIOT(12)=1.32
RATIOT(13)=1.41
RATIOT(14)=1.51
RATIOT(15)=1.62
RATIOT(16)=1.75
RATIOT(17)=1.91
RATIOT(18)=2.15
RATIOT(19)=2.60
RATIOT(20)=5.00
NN=INT(5*TTP/DELT)
XNN=5*TTP/DELT
IF(XNN.GT.FLOAT(NN))NN=NN+1
IF(NN.LE.N)GO TO 27
NPLUS=N+1
DO 28 I=NPLUS,NN
28 T(I)=T(I-1)+DELT
27 CONTINUE
DO 12 I=1,NN
TTP=T(I)/TTP
IF(TTP.GE.5.)GO TO 15
IFLAG=1
DO 13 J=1,20
IF(TTP.LE.RATIOT(J))GO TO 13
IFLAG=IFLAG+1
13 CONTINUE
IF(IFLAG.GT.1)GO TO 14
QQT(I)=(TTP/RATIOT(1))*RATIOQ(1)
GO TO 12
14 QQT(I)=((TTP-RATIOT(IFLAG-1))/(RATIOT(IFLAG)-RATIOT(IFLAG-1)))
QQT(I)=QQT(I)*(RATIOQ(IFLAG)-RATIOQ(IFLAG-1))
QQT(I)=QQT(I)+RATIOQ(IFLAG-1)
GO TO 12
15 QQT(I)=1.0
12 CONTINUE
DO 16 I=1,NN
IF(I.EQ.1)GO TO 25
DELTA(I)=(QQT(I)-QQT(I-1))
GO TO 16
25 DELTA(I)=QQT(I)
16 CONTINUE
SUMDEL=0.0
DO 21 I=1,NN
21 SUMDEL=SUMDEL+DELTA(I)
DO 23 I=1,NN
23 DELTA(I)=DELTA(I)/SUMDEL
WRITE(6,8)
WRITE(7,8)
8 FORMAT(/,10X,'UNIT HYDROGRAPH',/,3X,'TIME(HR)',10X,'DIMENSIONLESS
*ORDINATES'/)
DO 22 I=1,NN
WRITE(6,17)T(I),DELTA(I)
WRITE(7,17)T(I),DELTA(I)
17 FORMAT(1X,F10.3,17X,F10.3)
22 CONTINUE
RETURN
END
SUBROUTINE ROUTE
COMMON/A1/DELT,QU,N,P,TD,CN,II
COMMON/A2/KT,TP,RP,WP,K
COMMON/A4/T(250),R(250),RE(250),RER(250)
COMMON/A8/AREA,L,Y,NN,NF
COMMON/A9/DELTA(250),CNM
COMMON/A10/DELP(250),QA(250),TM(250),QM(250)
COMMON/A11/SUBPT1,SUBPT2,SUBPT3,SUBPT4
COMMON/A12/TL
DIMENSION DD(250)
MM=NN+NF-1

```

```

NPLUS=NF+1
IF(NPLUS.LE.1)NPLUS=2
IF(MM.LE.1)MM=3
DO 10 I=NPLUS,MM
TM(I)=TM(I-1)+DELT
10 DELP(I)=0.0
NPLUS=NN+1
IF(MM.LT.150)GOTO 25
MM=150
NPLUS=149
25 CONTINUE
DO 11 I=NPLUS,MM
11 DELTA(I)=0.0
WRITE(6,17)
WRITE(7,17)
17 FORMAT(/,1X,'FLOOD HYDROGRAPH COMPUTATION
*' /10X,' BY CN - INFIL MODEL ',/)
WRITE(6,20)
WRITE(7,20)
20 FORMAT(1X,65(1H*))
WRITE(6,21)
WRITE(7,21)
21 FORMAT(/,2X,'T(HR) = TIME AT THE END OF EACH STEP'/
*2X,'DD(MM/HR) = RATE OF RAINFALL EXCESS'/
*2X,'DELP(MM) = INCREMENTAL DEPTH OF EXCESS RAINFALL'/
*2X,'QA(M3/S) = DISCHARGES COMPUTED'/
*2X,'QM(M3/S) = DISCHARGES OBSERVED (IF SO)')///)
WRITE(6,40)
WRITE(7,40)
22 FORMAT(1X,F5.2,4X,F8.2,4X,F8.2,4X,F8.3,4X,F8.3)
23 FORMAT(1X,F5.2,4X,F8.2,4X,F8.2,4X,F8.3)
24 FORMAT(1X,60(1H-))
WRITE(6,24)
WRITE(7,24)
IF(SUBPT3.EQ.0)GO TO 29
IF(II.LT.MM)GO TO 29
DO 27 I=II,MM
27 QM(I)=QM(II)
29 CONTINUE
DO 14 I=1,MM
14 DD(I)=DELP(I)/DELT
C CONVOLUTION OPERATION
DO 13 I=1,MM
QA(I)=0.0
DO 13 J=1,I
QA(I)=QA(I)+DELP(J)*DELTA(I-J+1)
13 CONTINUE
C CONVERT FROM MM/HR TO M3/S
DO 30 I=1,MM
QA(I)=(AREA/3.6)*QA(I)/DELT
QA(I)=QA(I)*CNM
30 CONTINUE
IF(SUBPT3.EQ.0)GO TO 31
40 FORMAT(2X,'T(HR)',3X,'DD(MM/HR)',3X,'DELP(MM)',4X,'QA(M3/S)',4X,
*'QM(M3/S)')/ )
DO 2 I=1,MM
2 QM(I)=QM(I)-QM(1)
DO 9 I=1,MM
IJ=K+I
QM(I)=QM(IJ)
IF(QM(I).LE.0.0)QM(I)=0.0
WRITE(6,22)TM(I),DD(I),DELP(I),QA(I),QM(I)
WRITE(7,22)TM(I),DD(I),DELP(I),QA(I),QM(I)
9 CONTINUE
GO TO 42
31 CONTINUE
DO 8 I=1,MM
WRITE(6,23)TM(I),DD(I),DELP(I),QA(I)
WRITE(7,23)TM(I),DD(I),DELP(I),QA(I)
8 CONTINUE
42 CONTINUE
RETURN
END
SUBROUTINE GODFI
COMMON/A1/DELT,QU,N,P,TD,CN,II
COMMON/A4/T(250),R(250),RE(250),RER(250)
COMMON/A7/CUMP(250)

```

```

COMMON/A8/AREA,L,Y,NN,NF
COMMON/A10/DELP(250),QA(250),TM(250),QM(250)
COMMON/A13/CNAME(20)
INTEGER QU
REAL L
C
C      EVALUATION OF RESULTS
C
      II=N
      SQ2=0.
      SQM2=0.
      SUMC=0.
      SUMQ=0.
      SMQ=0.
      DO 1 I=1,N
1     SMQ=SMQ+QM(I)
      SMQ=SMQ/N
      DO 2 I=1,N
      SQ2=SQ2+(QM(I)-QA(I))**2
      SQM2=SQM2+(QM(I)-SMQ)**2
2     CONTINUE
      RI=(SQM2-SQ2)/SQM2
      PE=(SQRT(SQ2/N))/SMQ
      F1=0.
      F2=0.
      QMAX=0.
      QAMAX=0.
      DO 3 I=1,N
      F2=QA(I)-QM(I)
      F2=ABS(F2)*QM(I)
      F1=F1+F2
3     CONTINUE
      DO 4 I=1,N
      IF(QM(I).GT.QMAX)QMAX=QM(I)
      IF(QA(I).GT.QAMAX)QAMAX=QA(I)
      SUMQ=SUMQ+QM(I)
      SUMC=SUMC+QA(I)
4     CONTINUE
      DEV=(F1*200.0)/(FLOAT(N)*(QMAX*QMAX))
      PEAK=(QMAX-QAMAX)/QMAX
      PEAK=PEAK*100.
      TVOL=(SUMQ-SUMC)/SUMQ
      TVOL=TVOL*100.
C      PRINT EVALUTION OF RESULTS
      WRITE(6,20)
20     FORMAT(2X//)
      WRITE(6,21)
      WRITE(7,21)
21     FORMAT(2X,'THE GOODNES OF FITTING CRITERIA',/2X,32(1H-)/)
22     FORMAT(2X,'COEFFICIENT OF DETERMINATION RE=',F7.2/
*2X,'COEFFICIENT OF VARIATION PE=',F7.2/
*2X,'SCHULTZE HYDROL.DEVIATION DEV=',F7.2/
*2X,'PEAK ERROR PEAK(PERC)=',F7.2/
*2X,'TOTAL RUNOFF ERROR TVOL(PERC)=',F7.2//)
      WRITE(6,22)RI,PE,DEV,PEAK,TVOL
      WRITE(6,20)
      WRITE(7,22)RI,PE,DEV,PEAK,TVOL
      WRITE(7,20)
      RETURN
      END

```

KIN - data TŘEBSÍN

```

$DEBUG
$NOTRUNCATE
      PROGRAM CPLANEKS
C      PROGRAM PLANEK SERVES FOR DESIGN DISCHARGES COMPUTATION
C      IT IS A VERSION OF CPLANE WITH SUMMATION OF PARTIAL PLANES
C      AND SEGMENTS AS AN INPUT TO CSTREAM OR MUSK PROGRAMMES
C      NAME OF FILE: CPLANEKS.FOR
COMMON/A1/DELT,PP,N,P,TD,CN,II,IT,IL,IK,TTM,TDR
COMMON/A4/T(150),R(150),RE(150),RER(150),QAB(150)
COMMON/A5/SO(10),DLN(10),WI(10),WID(10),AK1(10),ALPHA(10),AM(10)
COMMON/A8/AREA,L,Y,NN,NF,TIM,DT
COMMON/A9/FRNM(10),FRIC(10),DX(10),PBAC(10),AR(10),AL(10),PR(10)
COMMON/A10/DELP(150),QA(150),TM(150),QM(150),TIND(150),QAA(150)

```

```

COMMON/A12/H(11,150),VE(11,150),TAU(11,150),VSTR(11,150)
COMMON/A13/CNAME(20)
COMMON/A14/NPL,QAW(20,150),qcw(20,150)
INTEGER PP,PPP,PM,FLAG,J,I,IT,IL,N,II,NPL,K
REAL HKIN,HBAC,HCEN,HFOR,RO,GA,DEL,TIME,QL,AK11,AM1,PBAC1,PR1
REAL PEFF,EFR,Q0
DIMENSION QC(150)
PARAMETER(PM=150)
PARAMETER(PPP=10)
C PARAMETER(PP=3)
C CHARACTER*12 NDFILE
C CHARACTER*30 START
C CHARACTER*8 FNAME$
C
C PURPOSE: TO SOLVE THE KINEMATIC WAVE EQUATION FOR OVERLAND FLOW
C ON A CASCADE OF PLANES UNDER RAINFALL, USING THE SECOND
C ORDER EXPLICIT FINITE-DIFFERENCE LAX-WENDROFF SCHEME.
C THE BACKWARDS FINITE DIFFERENCE SCHEME IS USED AT THE
C END OF EACH PLANE. THE VARIABLE WIDTHS OF PLANES CAN
C ALSO BE USED FOR A SEGMENT FLOW SIMULATION.
C
C
RO=1000.0
GA=9.81
K=0
OPEN(UNIT=5,FILE='TREB9.DTA',STATUS='OLD')
OPEN(UNIT=7,FILE='TREB9.OUT',STATUS='UNKNOWN')
OPEN(UNIT=1,FILE='NDFILE',STATUS='UNKNOWN')
C INPUT OF DATA
C READ INITIAL DISCHARGE (for design discharges obviously Q0=0.0)
97 FORMAT(F10.4)
READ(5,97)Q0
C beginning of cycle data reading
100 PRINT *, 'GIVE THE NAME OF DATA FILE: '
C READ '(A12)',NDFILE
C READ IN DATA FILE
99 FORMAT(20A4)
READ(5,99)CNAME
98 FORMAT(2X,60(1H*))
C READ NUMBER OF GEOMETRIC ELEMENTS (PLANES, SEGMENTS) TO BE SUMMED
READ(5,105)NPL
C READ THE NUMBER OF PLANES IN A CASCADE, PP
READ(5,105)PP
C READ THE PLANE PARAMETERS:
C SLOPE SO(J), LENGTH DLN(J), WIDTH WID(J), OBSTACKLES AR(J),
C MANNING ROUGHNESS FRNM(J), FRICTION FRIC(J), FLOW TYPE AM(J).
101 FORMAT(7F10.4)
DO 2 J=1,PP
READ(5,101)SO(J),DLN(J),WID(J),AR(J),FRNM(J),FRIC(J),AM(J)
2 CONTINUE
102 FORMAT(2F10.2)
103 FORMAT(10F8.3)
READ(5,102)DELT,TTM
RCO=3600.
105 FORMAT(I3)
READ(5,105)N
READ(5,103)(RER(I),I=1,N)
TDLN=0.
SI=0.
DO 17 I=1,N
RER(I)=RER(I)/0.01665 ! 1 min
C RER(I)=RER(I)/0.08325 ! 5 min
C RER(I)=RER(I)/0.1665 ! 10 min
C RER(I)=RER(I)/0.5 ! 30 min
SI=SI+RER(I)
17 CONTINUE
SI=SI*(DELT/RCO)
DO 18 J=1,PP
18 TDLN=TDLN+DLN(J)
PAR=(TDLN/1000.)*(WID(1)/1000.)
C TRANSFER FROM MM/HR TO M/S
RER(1)=RER(1)/RCO/1000.0
T(1)=DELT
DO 3 I=2,N
RER(I)=RER(I)/RCO/1000.0
T(I)=T(I-1)+DELT
3 CONTINUE

```

```

112 FORMAT(I1)
    READ(5,112)NEXT
C
4 PRINT *, 'GIVE TIME INCREMENT (DT):      '
    READ *,DT
C PRINT *, 'GIVE TIME INTERVAL FOR WRITING INTERMEDIATE RESULTS:      '
C PRINT *, 'RECOMMENDATION:TIM SHOULD EQUAL PREFERABLY TO DELT, '
C PRINT *, 'BUT NOT NECESSARILLY'
C READ *,TIM
C
C INITIAL CONDITIONS
TIM=DELT
TIME=TIM
TTIM=30.0
IL=11
IK=0
DO 6 J=1,PP
FRNM(J)=FRNM(J)*1.0      !
AL(J)=ATAN(SO(J))
AK1(J)=SQRT(8.0*GA*SO(J)/FRIC(J))
ALPHA(J)=SQRT(SO(J))/FRNM(J)
DX(J)=DLN(J)/10.0
FRNM(J)=FRNM(J)/1.0      !
IF(AR(J).LE.1.0)THEN
WI(J)=1.0-AR(J)
ELSE
WI(J)=1.0*AR(J)
ENDIF
DO 5 I=1,IL
H(J,I)=0.0
5 CONTINUE
6 CONTINUE
IT=0
C WRITING INITIAL DATA
113 FORMAT(2X, 'NAME OF CATCHMENT: ', 2X, 20A4/2X, 60(1H*))
    WRITE(6,113)CNAME
    WRITE(7,113)CNAME
107 FORMAT(2X, 'OUTPUT OF SUBROUTINE CPLANE: '/2X, 28(1H*))
C WRITE(6,107)
C WRITE(7,107)
104 FORMAT(2X, 'SIMULATION OF OVERLAND FLOW IN THE CASCADE OF PLANES' /
*2X, '(EXPLICIT FINITE DIFFERENCE SCHEME OF LAX-WENDROFF): '/')
C
C PRINTING DETAILED SUBHEADINGS
106 FORMAT(2X, 'INTERIM RESULTS OF SIMULATION: '/2X, 30(1H-))
C WRITE(6,104)
C WRITE(7,104)
108 FORMAT(2X, 'NUMBER OF PLANES IN A CASCADE: ', 3X, I2)
109 FORMAT(2X, 'DESCRIPTION OF PLANES: '/2X, 22(1H-)/2X, 'SLOPES(-)', 5X,
* 'LENGTHS(M)', 5X, 'WIDTHS(M)', 5X, 'MAN.ROUGHNESS', 5X, 'M-FLOWTYPE')
110 FORMAT(4X, F6.3, 5X, F9.3, 6X, F9.3, 8X, F7.4, 10X, F7.4)
111 FORMAT(2X, 70(1H-))
C WRITE(6,108)PP
C WRITE(7,108)PP
    WRITE(6,109)
    WRITE(7,109)
    DO 16 J=1,PP
    WRITE(6,110)SO(J),DLN(J),WID(J),FRNM(J),AM(J)
    WRITE(7,110)SO(J),DLN(J),WID(J),FRNM(J),AM(J)
16 CONTINUE
    WRITE(6,111)
    WRITE(7,111)
C WRITE(6,106)
C WRITE(7,106)
C
C START OF SIMULATION
DO 1 TD=DT,TTM,DT
DO 7 I=1,N
    IF(TD.LE.T(1))THEN
        EFR=RER(1)
    ELSE IF(TD.GT.T(N))THEN
        EFR=0.0
    ELSE IF(TD.LE.T(I).AND.TD.GT.T(I-1))THEN
        EFR=RER(I)
        GO TO 7
    ENDF
7 CONTINUE

```

```

C     PREPARATION OF EFFECTIVE RAIN (PEFF) FOR EACH SLOPE
DO 8 J=1,PP
PEFF=EFR*WI(J)
DDT=TD-DT
IF(DDT.LT.0.001) PR(J)=PEFF
PBAC(J)=PR(J)
PR(J)=PEFF
8 CONTINUE
C     START OF SPACE SIMULATION FOR DIFFERENT SLOPES
DO 10 J=1,PP
C     RESET VALUES FOR THE NEXT TIME STEP
I=2
HBAC=H(J,I-1)
HCEN=H(J,I)
HFOR=H(J,I+1)
C     COMPUTATION PROCEDURE
DO 9 I=2,IL
FLAG=0
IF(I.EQ.IL)FLAG=1
DX1=DX(J)
AK11=AK1(J)
ALP=ALPHA(J)
AM1=AM(J)
PBAC1=PBAC(J)
PR1=PR(J)
H(J,I)=HKIN(HBAC,HCEN,HFOR,DT,DX1,ALP,AK11,AM1,PBAC1,PR1,FLAG)
IF(H(J,I).LT.0.0)THEN
H(J,I)=0.
IF(H(PP,IL).LE.0.)THEN
IT=INT(TD+0.01)
CALL WRTR
PRINT *, '----->COMPUTATIONAL BLOCK '
GOTO 20
ENDIF
CONTINUE
ENDIF
C     RESET VALUES FOR CALCULATION ON THE NEXT GRIDPOINT
IF(I.GT.1.AND.I.LT.IL-1)THEN
HBAC=HCEN
HCEN=HFOR
HFOR=H(J,I+2)
ENDIF
9 CONTINUE
C     TEST OF STABILITY
IF(H(J,IL).GT.0.)DEL=DX(J)/(AM(J)*ALPHA(J)*H(J,IL)**(AM(J)-1.0))
IF(DT.GT.DEL.AND.DEL.GT.0.0)THEN
PRINT *, 'STABILITY CONDITIONS ARE NOT FULFILLED AT T '
ENDIF
IF(J.NE.PP)THEN
QL=ALPHA(J)*H(J,IL)**AM(J)*WI(J)*WID(J)
H(J+1,1)=(QL/WI(J+1)/WID(J+1)/ALPHA(J+1))**(1.0/AM(J+1))
ENDIF
10 CONTINUE
C     TEST WHETHER TO WRITE INTERIM RESULTS
C
11 IT=INT(TD+0.01)
IF(ABS(IT-TIME).LT.0.01)THEN
IF(H(PP,IL).GT.2E-6)THEN
CALL WRTR
ENDIF
TIME=TIME+TIM
ENDIF
C     END OF THE TIME CYCLE
1 CONTINUE
WRITE(6,'(A)')' '
WRITE(7,'(A)')' '
13 FORMAT(2X,'THE OVERLAND FLOW HYDROGRAPH: '/
*      2X,'ORDIN.NO.',6X,'TIME(HOURS)',2X,'DISCHARGE Q (L/S)')
WRITE(6,13)
WRITE(7,13)
DO 15 JK=1,IK
QAB(JK)=QAA(JK)*1000.0
WRITE(6,14)JK,TIND(JK),QAB(JK)
WRITE(7,14)JK,TIND(JK),QAB(JK)
14 FORMAT(2X,I3,8X,F10.3,8X,F10.3)
15 CONTINUE
C     MASS BALANCE CHECK

```



```

DELE=DELT/3600.
SQ=0.
DO 19 I=1,IK
19 SQ=SQ+QAA(I)
SQ=SQ*DELE*3.6/PAR
114 FORMAT(2X,'THE MASS BALANCE CHECK:'/2X,25(1H-),/
*2X,'TOTAL INFLOW DEPTH:',4X,F8.2,2X,'MM',/,
*2X,'TOTAL OUTFLOW DEPTH:',3X,F8.2,2X,'MM',///// )
WRITE(6,114)SI,SQ
WRITE(7,114)SI,SQ
GOTO 120
C SUMMATION OF OUTFLOW DEPTH FROM ALL AREAS (PLANES +SEGMENTS)
K=K+1
DO 25 I=1,IK
IF(K.EQ.1)THEN
QAW(K,I)=QAA(I)
ELSE
QAW(K,I)=QAW(K-1,I)+QAA(I)
ENDIF
25 CONTINUE
IF(NEXT.NE.0) GO TO 100
WRITE(6,116)
WRITE(7,116)
WRITE(6,117)K,NPL
WRITE(7,117)K,NPL
DO 26 I=1,IK
QAW(K,I)=Q0+QAW(K,I)
WRITE(6,115)I,TIND(I),QAW(K,I)
WRITE(7,115)I,TIND(I),QAW(K,I)
QC(I)=QAW(K,I)
26 CONTINUE
c qaw(k,i) values are up to now summative, which means that
c qaw(npl,i) values are equal to total values qc(i).
c next procedure computes outflow qcw from partial ares k=1,npl.
do 27 k=1,npl
do 27 i=1,ik
if(k.eq.1)then
qcw(k,i)=qaw(1,i)
else
qcw(k,i)=qaw(k,i)-qaw(k-1,i)
endif
27 continue
116 FORMAT(2X,'SUM OF THE HYDROGRAPH ORDINATES FROM ALL PARTIAL AREAS'
*/2X,60(1H*))
115 FORMAT(2X,I3,6X,F10.3,6X,F10.3)
117 FORMAT(2X,'K= ',I3,5X,'NPL= ',I3)
20 CONTINUE
C SCREEN CONTROL
READ '(A12)',NDFILE
C Data for graphs preparation
write*,'(A\)' NAME OF EVENT DATA:'
read*,'(A)'fname$
open(unit=8,file=fname$)
write(8,'(2x,i3,2f8.3)')(i,tind(i),qaa(i),i=1,ik)
c output data for all planes
c READ '(A12)',NDFILE
c write*,'(A\)' NAME OF DETAILED EVENT DATA:'
c read*,'(A)'fname$
c Pozor na formatovani tabulky- max.pocet sloupcu =12(12f6.3)
c write(8,'(1x,6f10.3)')(qcw(k,i),k=1,npl,i=1,ik) !
120 CONTINUE
CLOSE (8)
PRINT *, '*****END OF CASCADE OF PLANES*****'
if(next.eq.1)goto 100
12 STOP
END
FUNCTION HKIN(HBAC,HCEN,HFOR,DT,DX1,ALP,AK11,AM1,PBAC1,PR1,FLAG)
INTEGER FLAG
REAL HN1,HN2,HN3,HN4,DT,DX1,ALP,AK11,AM1,PBAC1,PR1
IF(FLAG.EQ.0)THEN
HN1=DT/2.0/DX1*(ALP*HFOR**AM1-ALP*HBAC**AM1-2.0*DX1*PBAC1)
HN2=(DT/DX1)**2.0/2.0*(ALP*AM1*HFOR** (AM1-1.)+ALP*AM1*HCEN**
*(AM1-1.))*(ALP*HFOR**AM1-ALP*HCEN**AM1-DX1*PBAC1)
HN3=(DT/DX1)**2.0/2.0*(ALP*AM1*HCEN** (AM1-1.)+ALP*AM1*HBAC**
*(AM1-1.))*(ALP*HCEN**AM1-ALP*HBAC**AM1-DX1*PBAC1)
HN4=DT/2.0*(PR1-PBAC1)
HKIN=HCEN-HN1+HN2-HN3+HN4

```

```

ELSE IF(FLAG.EQ.1)THEN
HN1=DT/DX1*(ALP*HFOR**AM1-ALP*HCEN**AM1-DX1*PBAC1)
HN2=(DT/DX1)**2.0/2.0*(ALP*AM1*HFOR**(AM1-1.)+ALP*AM1*HCEN**
*(AM1-1.))*(ALP*HFOR**AM1-ALP*HCEN**AM1-DX1*PBAC1)
HN3=(DT/DX1)**2.0/2.0*(ALP*AM1*HCEN**(AM1-1.)+ALP*AM1*HBAC**
*(AM1-1.))*(ALP*HCEN**AM1-ALP*HBAC**AM1-DX1*PBAC1)
HN4=DT/2.0*(PR1-PBAC1)
HKIN=HFOR-HN1+HN2-HN3+HN4
ENDIF
AK11=ALP
RETURN
END
SUBROUTINE WRTR
COMMON/A1/DELT,PP,N,P,TD,CN,II,IT,IL,IK,TTM,TDR
COMMON/A4/T(150),R(150),RE(150),RER(150),QAB(150)
COMMON/A5/SO(10),DLN(10),WI(10),WID(10),AK1(10),ALPHA(10),AM(10)
COMMON/A8/AREA,L,Y,NN,NF,TIM,DT
COMMON/A9/FRNM(10),FRIC(10),DX(10),PBAC(10),AR(10),AL(10),PR(10)
COMMON/A10/DELP(150),QA(150),TM(150),QM(150),TIND(150),QAA(150)
COMMON/A12/H(11,150),VE(11,150),TAU(11,150),VSTR(11,150)
INTEGER J,I,IL,IT,PM,PP
REAL X1,TK1,TK2,TK3,TK4
PARAMETER(PM=150)
C PARAMETER(PP=3)
C CHARACTER*12 NDFILE
C
C WRITING RESULTS
C
C LIMITS FOR CRITICAL SHEAR STRESS TK
TK1=0.0076
TK2=0.038
TK3=0.19
TK4=1.67
IT=IT/60
32 FORMAT(2X,'LIMITS FOR CRITICAL SHEAR STRESS TAUcr(Pa): '/
*2X,60(1H-))
WRITE(6,32)
WRITE(7,32)
33 FORMAT(2X,F7.4,3X,F7.4,3X,F7.4,3X,F7.4/)
WRITE(6,33)TK1,TK2,TK3,TK4
WRITE(7,33)TK1,TK2,TK3,TK4
DO 21 J=1,PP
DO 22 I=1,IL
VE(J,I)=ALPHA(J)*H(J,I)**(AM(J)-1.)
TAU(J,I)=9810.0*SO(J)*H(J,I)
X1=9.81*SO(J)*H(J,I)
VSTR(J,I)=SQRT(X1)
22 CONTINUE
QA(J)=ALPHA(J)*H(J,IL)**AM(J)*WI(J)*WID(J)
C PRINT INTERIM RESULTS
C DISCHARGE AT L IN L/S
23 FORMAT(2X,'J=' ,I2,' T= ' ,I5,/
*2X,' H(J,I): ' ,6F8.4,/
*2X,' VE(J,I): ' ,6F8.4)
34 FORMAT(2X,' TAU(J,I): ' ,6F8.4/
*2X,'VSTR(J,I): ' ,6F8.4/2X,60(1H-))
WRITE(6,'(2X,A,I2,3X,A,I4)') 'PLANE ',J, 'AT TIME ',IT
WRITE(7,'(2X,A,I2,3X,A,I4)') 'PLANE ',J, 'AT TIME ',IT
WRITE(6,23)J,IT,(H(J,I),I=1,11,2),(VE(J,I),I=1,11,2)
WRITE(7,23)J,IT,(H(J,I),I=1,11,2),(VE(J,I),I=1,11,2)
WRITE(6,34)(TAU(J,I),I=1,11,2),(VSTR(J,I),I=1,11,2)
WRITE(7,34)(TAU(J,I),I=1,11,2),(VSTR(J,I),I=1,11,2)
21 CONTINUE
C WRITE(6,'(I5,10F6.3)')IT,(QA(J),J=1,PP)
C WRITE(7,'(A)')' '
C WRITE(7,'(I5,10F6.3)')IT,(QA(J),J=1,PP)
C WRITE(6,27)
C WRITE(7,27)
27 FORMAT(2X,60(1H-))
C WRITE(6,'(A)')' '
C WRITE(7,'(A)')' '
29 SIT=0.0
DO 30 J=1,PP
IF(J.EQ.PP) THEN
JK=INT(J/PP)
IK=IK+JK
QAA(IK)=QA(J)

```

```
SIT=SIT+IT
TIND(IK)=SIT/60.0
WRITE(6,24)IK,IT,TIND(IK),QAA(IK)
WRITE(7,24)IK,IT,TIND(IK),QAA(IK)
24 FORMAT(3X,I4,9X,I4,6X,F7.2,7X,F7.3)
ENDIF
30 CONTINUE
WRITE(6,27)
WRITE(7,27)
31 RETURN
END
```